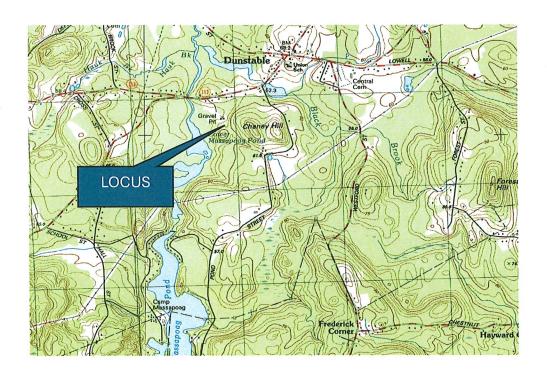


# STORMWATER ANALYSIS

MCO COTTAGE RENTALS
PLEASANT STREET
Dunstable, MA

APPLICANT: MCO Cottage Rentals P.O. Box 372 | Harvard, MA 01451



REPORT PREPARED BY: Haley Ward, Inc.

510 Mechanic Street | Leominster, MA 01453

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# 1.0 DRAINAGE NARRATIVE

#### 1.0 NARRATIVE

#### **1.1 INTRODUCTION**

As part of the submittal of a site plan for a new residential development off of Pleasant Street in Dunstable, MA, Haley Ward, Inc., has prepared this stormwater system analysis pursuant to the Town of Dunstable rules and regulations. The purpose of this report is to compare the pre-construction drainage conditions to the post-construction drainage conditions relative to increases in runoff generally associated with the increase of developed area.

The proposed project consists of the construction of a new 48-unit residential development with associated roadways, driveways, parking areas, utilities, and stormwater management devices. For a complete description of the site plan refer to a set of plans, entitled "Site Development Plan for MCO Cottage Rentals located at 164 Pleasant Street in Dunstable, Massachusetts" dated April 7, 2025 prepared by Haley Ward, Inc., 510 Mechanic Street, Leominster, MA 01453.

#### 1.2 METHOD OF ANALYSIS

The enclosed hydrologic calculations utilize the runoff estimating techniques developed by the USDA Soil Conservation Service (SCS). The following publications were used in the preparation of this report:

- 1. "Urban Hydrology for Small Watersheds" 1
- 2. "National Engineering Handbook, Hydrology, Section 4" (NEH-4)<sup>2</sup>

Using SCS publications and other texts on surface water hydrology, in conjunction with drainage software "HydroCAD" developed by Applied Microcomputer Systems<sup>3</sup>, Whitman & Bingham Associates has calculated peak rates of runoff relative to the subject site. The drainage software program "HydroCAD" calculates peak rates of runoff similarly to the computer program known as "Computer Programs for Project Formulations Hydrology, Technical Release Number 20" (TR-20), developed by SCS.

Times of concentration are computed by using the "Upland Method" which calculates times of concentrations utilizing land use and slope in conjunction with other land use variables. Time of concentration is the time required for a droplet of water to travel from the most hydraulically distant point of the subwatershed to the design point. It is computed by summing the times it takes the water to travel through the different components of the subwatershed drainage system.

As a result of urbanization of an area (increase of impervious area), time of concentration will be decreased since velocity of overland flow will be increased. Therefore, as time of concentration is decreased, the hydrographic peak will be increased for that particular subwatershed. The time of concentration (Tc) is a major hydrograph shape parameter. The hydrograph peak for a subwatershed area is attenuated and shifted to the right with an increase in Tc. The "upland method" translates to more accurate Tc's than other general methods.

#### **1.3 SOILS**

Generally, soils are classified from A through D, with A being very pervious (i.e. gravels, etc.) and D being very impervious (i.e. clays, muck, peat, etc.).

The four (4) soil classifications as defined by SCS are:

Group A	Generally have low runoff potential and have high infiltration
	rates. This group includes mainly sands and gravels consisting
	of deep, well to excessively drained media (sand, loamy sand,
	or sandy loam).

Group B Soils in this classification have moderate infiltration rates and contain well drained soils with moderately fine to moderately coarse textures (silt loam or loam).

Group C These soils have low infiltration rates and generally consist of a soil layer that prevents downward movement of water to the water table (sandy, clay, loam).

Group D Soils under this group have high runoff potential since they are chiefly clay soils with high expansive capability. High water table, clay layers, nearly impervious material are all associated with this classification (clay loam, silty clay loam, sand clay, silty clay, or clay).

Soils throughout the subject site were classified by the Soil Conservation Service (SCS) as the following soil groups:

Soil Group	Hydrologic Runoff Class
Hinckley loamy sand	Ā
Scarboro mucky fine sandy loam	A/D
Freetown muck	D
(wetland áreas)	

These types of soils are described by the SCS in the *preceding paragraph*.

A series of on-site test pits were performed by Haley Ward, inc. Soils were found to be well-drained sands. Therefore, the analysis utilized an infiltration rate of 8.27 inches per hour per Table 2.3.3 1982 Rawls Rates as found in the Massachusetts Stormwater handbook, Volume 3.

#### **1.4 RUNOFF CURVE NUMBERS**

There are several major factors affecting the amount of runoff that can be expected from a particular land use. They include treatment, cover type, hydrologic condition, and antecedent runoff condition.

Hydraulic condition represents the effects of cover type and treatment on infiltration and runoff. Density of plant cover and forest litter heavily influences this condition. Good hydraulic condition indicates a low run-off potential due to heavy ground cover while poor hydraulic condition indicates poor cover (i.e. bare soil, etc.) and is indicative of high runoff potential.

As the curve number increases the amount of expected runoff is also increased. A curve number of 30 indicates a low potential for runoff while a number of 90 indicates a high potential. Curve numbers were selected for the various soil types and land uses found on the site in accordance with the following table. Weighted curve numbers for each subwatershed were determined by land area of the different soil types.

The SCS runoff curve numbers used in all watershed modeling contained in this report are based upon the following Hydrologic Soil Group and their following land uses:

SURFACE TYPE	HY	HYDROLOGICAL SOIL GROUP				
	A	В	$\mathbf{C}$	D		
		CURV	E NUMBEI	RS		
Pavement	98	98	98	98		
Grass/Lawn	39	61	74	80		
Wooded	30	55	70	77		
Woods – Grass Combination (good cover)	32	58	72	79		
Gravel - Dirt Driveways	75	83	88	90		
Residential 1/2 Acre Zoning	54	70	80	85		
Residential 1 Acre Zoning	51	68	79	84		
Residential 2 Acre Zoning	46	65	77	82		
Pasture (grazing purposes)	39	61	74	80		
Bare Soil Surfaces (little or no vegetation)	77	86	91	94		
Residential districts, 2-acre lots	46	65	77	82		
Open space, grass cover 50% to 75%	49	69	79	84		

#### 1.5 DESIGN CRITERIA

This drainage analysis was developed utilizing storm frequencies and the corresponding rainfall depths were compiled from NOAA Atlas 14, Volume 10, Versions and are as follows:

Storm Frequency (years)	Rainfall Depth (inches)
100	7.48
25	5.86
10	4.81
2	3.12

The stormwater management network has been designed to handle up to and including the 100-year design storm. Calculations relative to the 2, 10, 25, and 100-year storms are provided in section 2.0 herein.

#### **1.6 HYDROLOGICAL DESCRIPTIONS**

#### **1.6.1 Pre-Development**

The subject site is located on the southerly side of Pleasant Street in Tyngsborough, Massachusetts. The site is bounded to the north by the United States Post office and a parcel of undeveloped property along Pleasant Street. The parcel is bounded to the west by Salmon Brook and its associated wetland system. The parcel is bounded to the south by residentially developed areas and o the east by undeveloped property.

The topography of the site consists of mostly gentle slopes within the area proposed for development. There are steep slopes to the east and to the north entering the property that are indicative of earth removal activities.

The subject site drains in a westerly manner towards Salmon Brook and the adjacent wetland system.

A portion of the property lies within the 100-year flood hazard zone as referenced on FIRM panel 25017CO111E dated June 4, 2010.

The design point chosen for this analysis is the existing wetland system located at the westerly side of the property. This design point was chosen to simulate the total change in peak rates of runoff at the limits of the proposed improvements. The composite hydrographs at these design points were chosen to compare peak rates of runoff for the 2, 10, and 25 and 100-year storm frequencies in order to show that there would be no negative drainage impacts to the abutting properties. For a more conservative approach, the pre-development watershed that was chosen encompasses only the area to the east of the wetland system where the proposed improvements will take place.

#### 1.6.2 Post-Development

As stated previously, the proposed project consists of the construction of a 48-unit residential development with associated roadways, driveways, parking areas, utilities, and stormwater management devices.

In order to mitigate the increase in the volume and the rate of stormwater runoff from the project, all stormwater runoff from proposed impervious areas will be connected to a subsurface stormwater infiltration or recharge system that will recharge the collected and treated stormwater into the underlying soil stratum.

Stormwater runoff from the roof areas of each building will be routed to individual roof recharge systems. These systems will be comprised of Stormtech SC-310 recharge chamber units or an approved equal. Refer to the submitted site plans for construction details.

All stormwater runoff from the proposed road, driveway and parking areas will be collected by deep sump, hooded catch basins for initial treatment of the stormwater. The catch basins are connected, via a closed pipe system, to several subsurface recharge systems placed throughout the project site. The recharge system further treats the collected stormwater prior to discharge to the underlying soil stratum.

The recharge systems for the road, driveway and parking areas will be comprised of Stormtech SC-740 recharge chamber units or an approved equal. Refer to the submitted site plans for construction details. These systems are to be fitted with Stormtech's "Isolator Row technology to enhance the system's total suspended solids (TSS) removal efficiency. The stormwater volume directed to the proposed recharge systems is entirely contained within the chambers and do not overtop. Additionally, the site has been graded such that any stormwater runoff that may overtop the recharge systems due to a system failure, is directed away from the dwellings until a repair can be made.

The proposed recharge systems have been designed to accommodate all design storms up to and including the 100-year design storm.

Due to the proposed improvements of the subject site, the existing watershed was separated into smaller subwatersheds. These subwatersheds represent the tributary area(s) to the collection system and ultimately to the individual subsurface recharge area(s). Individual hydrographs were determined and were combined at the appropriate points to compare the total flows at the design point as mentioned in section 1.6.1 above.

#### **1.7 CONCLUSIONS**

The proposed site will encompass the design of the aforementioned storm water system which will mitigate the increase in peak rates of runoff as outlined below:

TABLE I Peak Flows at Design Point

Storm Event	<b>Existing Conditions</b>	<b>Proposed Conditions</b>
	(cfs)	(cfs)
100 yr. storm	18.73	11.28
25 yr. storm	4.51	2.32
10 yr. storm	0.88	0.49
2 yr. storm	0.00	0.00

As outlined in the table above, the proposed storm water management system has been designed so that no negative impact to abutting properties relative to increases in runoff will occur.

The storm water management system as outlined herein and as shown on the accompanying plans have the following positive values relative to stormwater management:

- A) The proposed stormwater network will collect stormwater runoff from impervious area(s) of the site and treat the collected stormwater runoff prior to discharge.
- B) Attenuation of the 2-year, 10-year, 25-year and 100-year design storm events will occur in the stormwater recharge system thus mitigating increased peak rates of storm water discharge.
- C) The proposed stormwater system has been designed in accordance with Town of Dunstable Regulations as well as the Massachusetts Department of Environmental Protection Stormwater Guidelines and Policies.

The subject drainage design was designed with good engineering practice and indicates that no negative impacts to abutting properties associated with stormwater will occur.



#### NOAA Atlas 14, Volume 10, Version 3 Location name: Dunstable, Massachusetts, USA\* Latitude: 42.6676°, Longitude: -71.4912° Elevation: 166 ft\*\*

\* source: ESRI Maps
\*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

#### PF tabular

PDS-	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) <sup>1</sup> Average recurrence interval (years)							ches)'		
Duration										1000
F and a	0.320	0.378	0.473	0,551	0.659	0.740	0.826	0.925	1.07	1.19
5-min		(0.305-0.468)	(0.380-0.586)	(0.441-0.687)	(0.508-0.855)	(0.556-0.979)	(0.600-1.13)	(0.630-1.29)	(0.697-1.54)	(0.755-1.74
10-min	<b>0.454</b> (0.367-0.560)	<b>0.536</b> (0.433-0.662)	<b>0.670</b> (0.539-0.831)	<b>0.781</b> (0.624-0.973)	<b>0.934</b> (0.719-1.21)	<b>1.05</b> (0.789-1.39)	<b>1.17</b> (0.850-1.60)	<b>1.31</b> (0.892-1.82)	<b>1.52</b> (0.988-2.18)	<b>1.69</b> (1.07-2.47)
15-min	<b>0.534</b> (0.432-0.659)	<b>0.630</b> (0.509-0.779)	<b>0.788</b> (0.634-0.977)	<b>0.919</b> (0.734-1.14)	<b>1.10</b> (0.846-1.43)	<b>1.23</b> (0.926-1.63)	<b>1.38</b> (1.00-1.89)	<b>1.54</b> (1.05-2.14)	<b>1.78</b> (1.16-2.56)	<b>1.99</b> (1.26-2.91)
30-min	<b>0.731</b> (0.591-0.903)	<b>0.864</b> (0.698-1.07)	<b>1.08</b> (0.870-1.34)	<b>1.26</b> (1.01-1.57)	<b>1.51</b> (1.16-1.96)	<b>1.69</b> (1.27-2.24)	<b>1.89</b> (1.37-2.59)	<b>2.12</b> (1.44-2.94)	<b>2.45</b> (1.60-3.52)	<b>2.73</b> (1.73-3.99)
60-min	<b>0.929</b> (0.751-1.15)	<b>1.10</b> (0.886-1.36)	<b>1.37</b> (1.10-1.70)	<b>1.60</b> (1.28-2.00)	<b>1.92</b> (1.48-2.49)	<b>2.15</b> (1.62-2.85)	<b>2.40</b> (1.75-3.29)	<b>2.69</b> (1.83-3.75)	<b>3.12</b> (2.03-4.48)	<b>3.47</b> (2.20-5.08)
2-hr	<b>1.16</b> (0.948-1.43)	<b>1.40</b> (1.14-1.72)	1.79 (1.45-2.21)	<b>2.11</b> (1.70-2.62)	<b>2.56</b> (1.98-3.31)	<b>2.89</b> (2.19-3.81)	<b>3.24</b> (2.38-4.45)	<b>3.67</b> (2.51-5.08)	<b>4.32</b> (2.82-6.16)	<b>4.88</b> (3.10-7.08)
3-hr	<b>1.33</b> (1.09-1.63)	<b>1.62</b> (1.32-1.98)	<b>2.08</b> (1.69-2.56)	<b>2.47</b> (1.99-3.04)	<b>3.00</b> (2.34-3.87)	<b>3.39</b> (2.58-4.47)	3.82 (2.82-5.24)	<b>4.34</b> (2.97-5.99)	<b>5.14</b> (3.36-7.31)	<b>5.83</b> (3.71-8.44)
6-hr	<b>1.68</b> (1.39-2.04)	<b>2.06</b> (1.69-2.50)	<b>2.67</b> (2.18-3.25)	<b>3.17</b> (2.58-3.89)	<b>3.87</b> (3.03-4.96)	<b>4.38</b> (3.36-5.74)	<b>4.94</b> (3.67-6.74)	<b>5.63</b> (3.87-7.72)	<b>6.70</b> (4.39-9.46)	<b>7.62</b> (4.86-11.0)
12-hr	<b>2.12</b> (1.76-2.56)	<b>2.59</b> (2.14-3.13)	<b>3.35</b> (2.76-4.06)	<b>3.98</b> (3.26-4.85)	<b>4.85</b> (3.82-6.18)	<b>5.50</b> (4.23-7.15)	<b>6.20</b> (4.62-8.38)	<b>7.05</b> (4.86-9.59)	<b>8.38</b> (5.51-11.8)	<b>9.52</b> (6.09-13.6)
24-hr	<b>2.55</b> (2.13-3.06)	<b>3.12</b> (2.60-3.74)	<b>4.04</b> (3.35-4.86)	<b>4.81</b> (3.96-5.81)	<b>5.86</b> (4.64-7.41)	<b>6.64</b> (5.14-8.58)	<b>7.48</b> (5.60-10.1)	<b>8.52</b> (5.90-11.5)	<b>10.1</b> (6.68-14.1)	<b>11.5</b> (7.37-16.3)
2-day	<b>2.93</b> (2.46-3.48)	<b>3.59</b> (3.01-4.28)	<b>4.67</b> (3.90-5.58)	<b>5.57</b> (4.62-6.69)	<b>6.81</b> (5.43-8.55)	<b>7.72</b> (6.01-9.91)	<b>8.72</b> (6.56-11.6)	<b>9.93</b> (6.90-13.3)	<b>11.8</b> (7.81-16.3)	<b>13.4</b> (8.62-18.9)
3-day	<b>3.21</b> (2.71-3.81)	<b>3.92</b> (3.30-4.65)	<b>5.08</b> (4.26-6.04)	<b>6.04</b> (5.02-7.22)	<b>7.36</b> (5.88-9.20)	<b>8.33</b> (6.50-10.6)	<b>9.39</b> (7.08-12.5)	<b>10.7</b> (7.44-14.3)	<b>12.6</b> (8.39-17.4)	<b>14.3</b> (9.23-20.1)
4-day	<b>3.47</b> (2.94-4.10)	<b>4.21</b> (3.55-4.98)	<b>5.40</b> (4.54-6.41)	<b>6.40</b> (5.34-7.63)	<b>7.77</b> (6.22-9.67)	<b>8.78</b> (6.86-11.2)	<b>9.88</b> (7.45-13.0)	<b>11.2</b> (7.82-14.9)	<b>13.2</b> (8.76-18.1)	<b>14.9</b> (9.60-20.8)
7-day	<b>4.19</b> (3.56-4.92)	<b>4.96</b> (4.22-5.84)	<b>6.23</b> (5.26-7.35)	<b>7.27</b> (6.10-8.62)	<b>8.72</b> (7.01-10.8)	<b>9.79</b> (7.67-12.3)	<b>10.9</b> (8.24-14.3)	<b>12.3</b> (8.61-16.2)	<b>14.2</b> (9.50-19.4)	<b>15.9</b> (10.3-22.1)
10-day	<b>4.86</b> (4.15-5.70)	<b>5.66</b> (4.83-6.64)	<b>6.96</b> (5.91-8.19)	<b>8.04</b> (6.77-9.50)	<b>9.53</b> (7.68-11.7)	<b>10.6</b> (8.35-13.3)	<b>11.8</b> (8.90-15.3)	<b>13.1</b> (9.25-17.3)	<b>15.0</b> (10.1-20.5)	<b>16.6</b> (10.8-23.0)
20-day	<b>6.86</b> (5.90-7.98)	<b>7.72</b> (6.62-8.98)	<b>9.12</b> (7.79-10.6)	<b>10.3</b> (8.72-12.1)	<b>11.9</b> (9.62-14.4)	<b>13.1</b> (10.3-16.2)	<b>14.4</b> (10.8-18.2)	<b>15.6</b> (11.1-20.4)	<b>17.3</b> (11.7-23.4)	<b>18.6</b> (12.1-25.6)
30-day	<b>8.52</b> (7.35-9.86)	<b>9.42</b> (8.12-10.9)	<b>10.9</b> (9.36-12.7)	<b>12.1</b> (10.3-14.2)	<b>13.8</b> (11.2-16.7)	<b>15.2</b> (11.9-18.6)	<b>16.5</b> (12.3-20.7)	<b>17.7</b> (12.6-23.0)	<b>19.3</b> (13.0-25.9)	<b>20.4</b> (13.3-28.0)
45-day	<b>10.6</b> (9.18-12.2)	<b>11.6</b> (10.0-13.3)	<b>13.2</b> (11.3-15.2)	<b>14.5</b> (12.4-16.8)	<b>16.3</b> (13.3-19.5)	<b>17.7</b> (14.0-21.6)	<b>19.1</b> (14.3-23.8)	<b>20.3</b> (14.5-26.3)	<b>21.8</b> (14.8-29.2)	<b>22.8</b> (14.9-31.1)
60-day	<b>12.3</b> (10.7-14.2)	<b>13.4</b> (11.6-15.4)	<b>15.0</b> (13.0-17.4)	<b>16.4</b> (14.1-19.1)	<b>18.4</b> (15.0-21.9)	<b>19.9</b> (15.8-24.1)	<b>21.3</b> (16.0-26.4)	<b>22.6</b> (16.2-29.1)	<b>24.1</b> (16.3-32.0)	<b>25.0</b> (16.4-34.0)

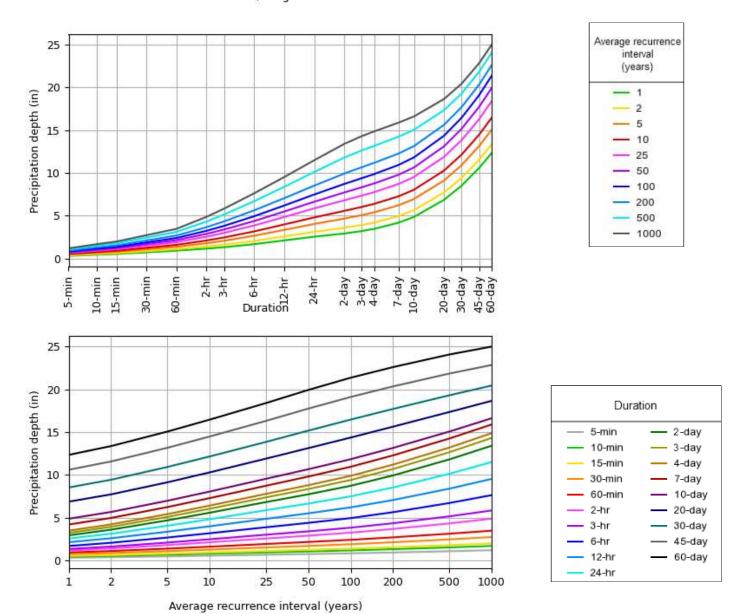
<sup>&</sup>lt;sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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#### PDS-based depth-duration-frequency (DDF) curves Latitude: 42.6676°, Longitude: -71.4912°



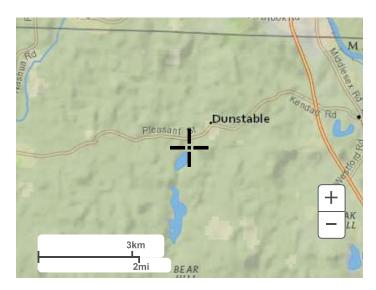
NOAA Atlas 14, Volume 10, Version 3

Created (GMT): Fri Mar 7 15:22:07 2025

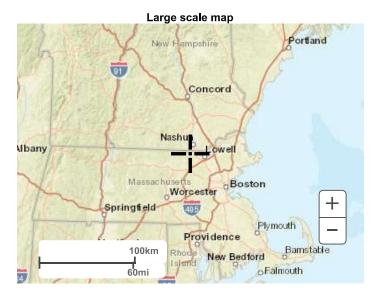
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#### Maps & aerials

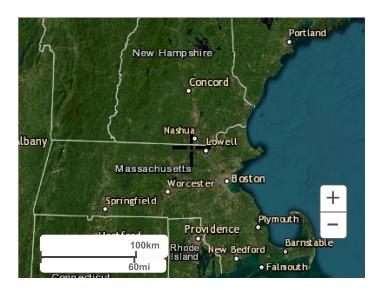
Small scale terrain



Large scale terrain Concord NEW HAMPSHIRE Albany Worcester \*Boston MASSACHUSETTS +Springfield Plymouth Cape Cod Bay Barnstable Providence 100km New Bedford 60mi Falmouth



Large scale aerial



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US Department of Commerce

National Oceanic and Atmospheric Administration

National Weather Service

National Water Center

1325 East West Highway

Silver Spring, MD 20910

Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

<u>Disclaimer</u>

# 2.0 HYDROLOGICAL CALCULATIONS



Pre-Development
Tributary Area to DP



**Design Point** 









Prepared by Haley Ward
HydroCAD® 10.20-6a s/n 00641 © 2024 HydroCAD Software Solutions LLC

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# **Rainfall Events Listing**

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	2-year Storm	Type III 24-hr		Default	24.00	1	3.12	2
2	10-year Storm	Type III 24-hr		Default	24.00	1	4.81	2
3	25-year Storm	Type III 24-hr		Default	24.00	1	5.86	2
4	100-year Storm	Type III 24-hr		Default	24.00	1	7.48	2

Prepared by Haley Ward
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# Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
11.832	46	2 acre lots, 12% imp, HSG A (8S)
9.446	49	50-75% Grass cover, Fair, HSG A (8S)
27.826	30	Woods, Good, HSG A (8S)
49.104	38	TOTAL AREA

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# Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
49.104	HSG A	8S
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.000	Other	
49.104		<b>TOTAL AREA</b>

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# **Ground Covers (all nodes)**

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
11.832	0.000	0.000	0.000	0.000	11.832	2 acre lots, 12% imp	8S
9.446	0.000	0.000	0.000	0.000	9.446	50-75% Grass cover, Fair	8S
27.826	0.000	0.000	0.000	0.000	27.826	Woods, Good	8S
49.104	0.000	0.000	0.000	0.000	49.104	TOTAL AREA	

# Pleasant Street Dunstable Pre-Development-202Type III 24-hr 2-year Storm Rainfall=3.12" Prepared by Haley Ward Printed 6/26/2025

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment8S: Pre-Development** Runoff Area=2,138,975 sf 2.89% Impervious Runoff Depth=0.00"

Flow Length=1,285' Tc=23.5 min CN=38 Runoff=0.00 cfs 0.000 af

Reach DP: Design Point Inflow=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af

Total Runoff Area = 49.104 ac Runoff Volume = 0.000 af Average Runoff Depth = 0.00" 97.11% Pervious = 47.684 ac 2.89% Impervious = 1.420 ac HydroCAD® 10.20-6a s/n 00641 © 2024 HydroCAD Software Solutions LLC

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# Summary for Subcatchment 8S: Pre-Development Tributary Area to DP

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

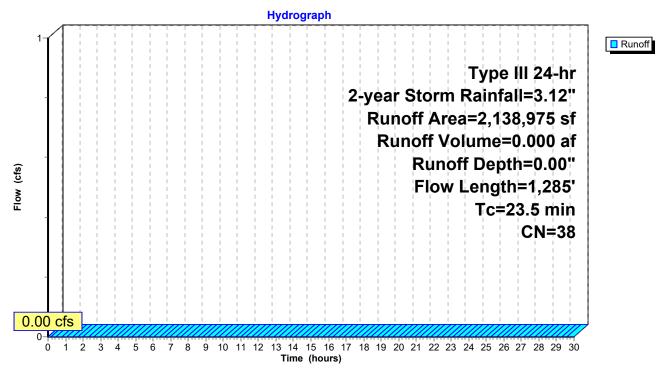
Routed to Reach DP : Design Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-year Storm Rainfall=3.12"

_	Α	rea (sf)	CN E	Description Description								
	5	15,400	46 2	acre lots,	acre lots, 12% imp, HSG A							
	4	11,475	49 5	50-75% Grass cover, Fair, HSG A								
_	1,2	12,100	30 V	Woods, Good, HSG A								
	2,1	38,975	38 V	Veighted A	verage							
	2,0	77,127	9	7.11% Per	vious Area							
		61,848	2	89% Impe	ervious Area	a e e e e e e e e e e e e e e e e e e e						
	_											
	Tc	Length	Slope	Velocity	Capacity	Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	15.1	50	0.0500	0.06		Sheet Flow,						
						Woods: Dense underbrush n= 0.800 P2= 3.12"						
	5.4	725	0.2000	2.24		Shallow Concentrated Flow,						
						Woodland Kv= 5.0 fps						
	3.0	510	0.0300	2.79		Shallow Concentrated Flow,						
_						Unpaved Kv= 16.1 fps						
	23.5	1,285	Total									

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# **Subcatchment 8S: Pre-Development Tributary Area to DP**



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#### **Summary for Reach DP: Design Point**

[40] Hint: Not Described (Outflow=Inflow)

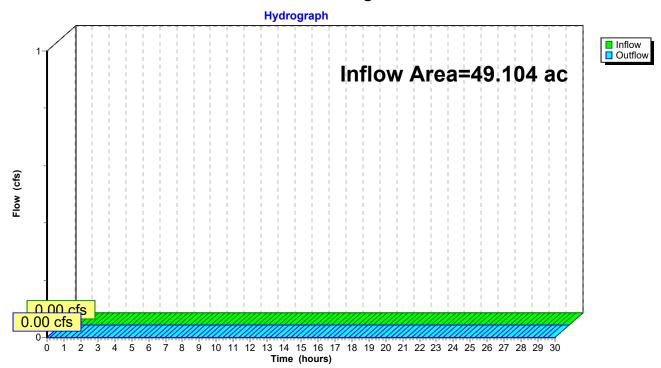
Inflow Area = 49.104 ac, 2.89% Impervious, Inflow Depth = 0.00" for 2-year Storm event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Outflow =  $0.00 \text{ cfs } \overline{@}$  0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

#### **Reach DP: Design Point**



# Pleasant Street Dunstable Pre-Development-20Type III 24-hr 10-year Storm Rainfall=4.81" Prepared by Haley Ward Printed 6/26/2025

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment8S: Pre-Development** Runoff Area=2,138,975 sf 2.89% Impervious Runoff Depth=0.13"

Flow Length=1,285' Tc=23.5 min CN=38 Runoff=0.88 cfs 0.548 af

Reach DP: Design Point Inflow=0.88 cfs 0.548 af Outflow=0.88 cfs 0.548 af

Total Runoff Area = 49.104 ac Runoff Volume = 0.548 af Average Runoff Depth = 0.13" 97.11% Pervious = 47.684 ac 2.89% Impervious = 1.420 ac

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## Summary for Subcatchment 8S: Pre-Development Tributary Area to DP

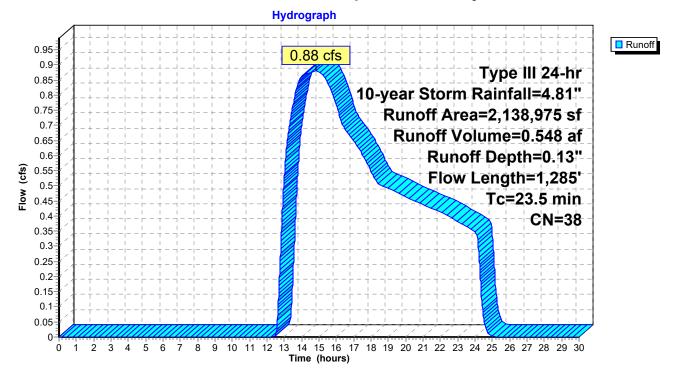
Runoff = 0.88 cfs @ 14.81 hrs, Volume= 0.548 af, Depth= 0.13"

Routed to Reach DP: Design Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Storm Rainfall=4.81"

	Α	rea (sf)	CN E	Description							
	5	15,400	46 2	acre lots, 12% imp, HSG A							
	4	11,475	49 5	0-75% Grass cover, Fair, HSG A							
	1,2	12,100	30 V	Woods, Good, HSG A							
	2,1	38,975	38 V	Veighted A	verage						
	2,0	77,127	9	7.11% Per	vious Area						
		61,848	2	89% Impe	ervious Area	a					
	Tc	Length	Slope	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	15.1	50	0.0500	0.06		Sheet Flow,					
						Woods: Dense underbrush n= 0.800 P2= 3.12"					
	5.4	725	0.2000	2.24		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	3.0	510	0.0300	2.79		Shallow Concentrated Flow,					
_						Unpaved Kv= 16.1 fps					
	23.5	1,285	Total								

# **Subcatchment 8S: Pre-Development Tributary Area to DP**



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#### **Summary for Reach DP: Design Point**

[40] Hint: Not Described (Outflow=Inflow)

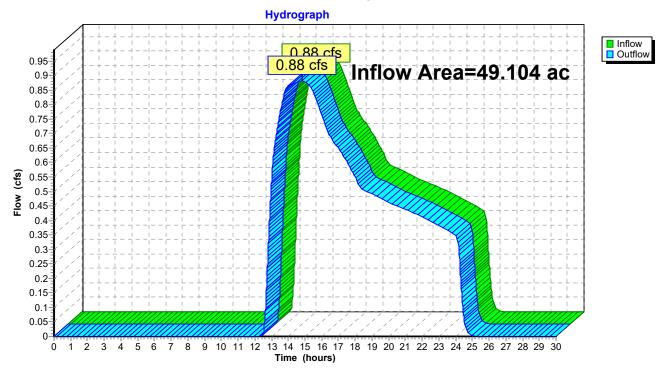
Inflow Area = 49.104 ac, 2.89% Impervious, Inflow Depth = 0.13" for 10-year Storm event

Inflow = 0.88 cfs @ 14.81 hrs, Volume= 0.548 af

Outflow = 0.88 cfs @ 14.81 hrs, Volume= 0.548 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

## **Reach DP: Design Point**



# Pleasant Street Dunstable Pre-Development-20 Type III 24-hr 25-year Storm Rainfall=5.86" Prepared by Haley Ward Printed 6/26/2025

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment8S: Pre-Development Runoff Area=2,138,975 sf 2.89% Impervious Runoff Depth=0.36"

Flow Length=1,285' Tc=23.5 min CN=38 Runoff=4.51 cfs 1.459 af

**Reach DP: Design Point**Inflow=4.51 cfs 1.459 af
Outflow=4.51 cfs 1.459 af

Total Runoff Area = 49.104 ac Runoff Volume = 1.459 af Average Runoff Depth = 0.36" 97.11% Pervious = 47.684 ac 2.89% Impervious = 1.420 ac

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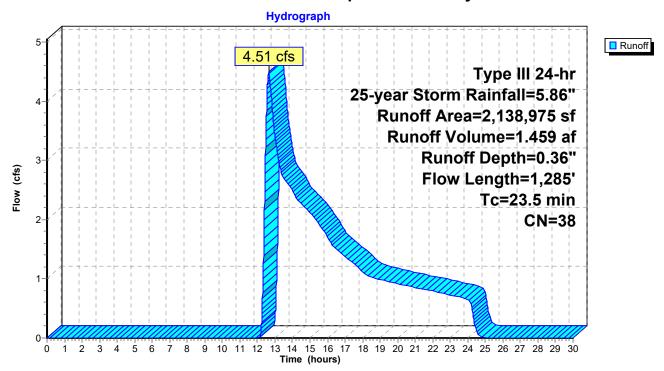
## Summary for Subcatchment 8S: Pre-Development Tributary Area to DP

Runoff = 4.51 cfs @ 12.66 hrs, Volume= 1.459 af, Depth= 0.36" Routed to Reach DP : Design Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-year Storm Rainfall=5.86"

A	rea (sf)	CN D	escription			
5	15,400	46 2	acre lots,	12% imp, F	HSG A	
4	11,475	49 5	0-75% Gra	ass cover, F	Fair, HSG A	
1,2	12,100	30 V	Voods, Go	od, HSG A		
2,1	38,975	38 V	Veighted A	verage		
2,077,127		-	97.11% Pervious Area			
	61,848	2	.89% Impe	ervious Area	a	
Tc	Length	Slope	Velocity	Capacity	Description	
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)		
15.1	50	0.0500	0.06		Sheet Flow,	
					Woods: Dense underbrush n= 0.800 P2= 3.12"	
5.4	725	0.2000	2.24		Shallow Concentrated Flow,	
					Woodland Kv= 5.0 fps	
3.0	510	0.0300	2.79		Shallow Concentrated Flow,	
					Unpaved Kv= 16.1 fps	
23.5	1,285	Total				

#### **Subcatchment 8S: Pre-Development Tributary Area to DP**



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#### **Summary for Reach DP: Design Point**

[40] Hint: Not Described (Outflow=Inflow)

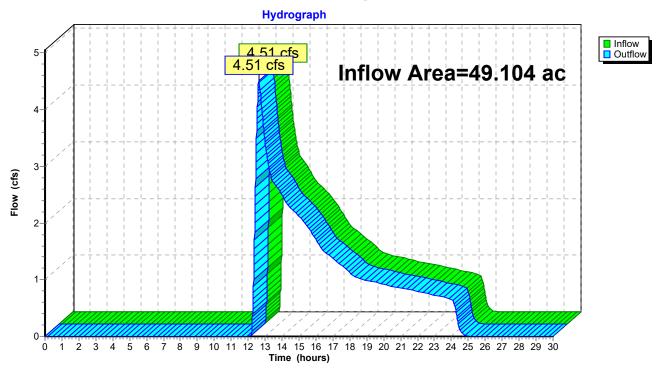
Inflow Area = 49.104 ac, 2.89% Impervious, Inflow Depth = 0.36" for 25-year Storm event

Inflow = 4.51 cfs @ 12.66 hrs, Volume= 1.459 af

Outflow = 4.51 cfs @ 12.66 hrs, Volume= 1.459 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

#### **Reach DP: Design Point**



Pleasant Street Dunstable Pre-Development-2 Type III 24-hr 100-year Storm Rainfall=7.48"
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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment8S: Pre-Development** Runoff Area=2,138,975 sf 2.89% Impervious Runoff Depth=0.87"

Flow Length=1,285' Tc=23.5 min CN=38 Runoff=18.73 cfs 3.544 af

Reach DP: Design Point Inflow=18.73 cfs 3.544 af Outflow=18.73 cfs 3.544 af

Total Runoff Area = 49.104 ac Runoff Volume = 3.544 af Average Runoff Depth = 0.87" 97.11% Pervious = 47.684 ac 2.89% Impervious = 1.420 ac

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## Summary for Subcatchment 8S: Pre-Development Tributary Area to DP

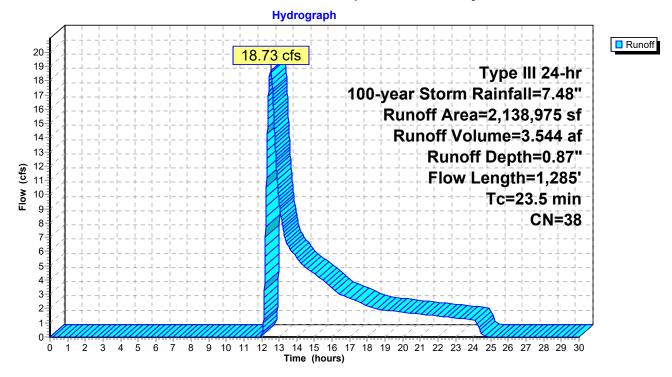
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Runoff = 18.73 cfs @ 12.51 hrs, Volume= 3.544 af, Depth= 0.87" Routed to Reach DP : Design Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Storm Rainfall=7.48"

A	rea (sf)	CN D	escription				
5	15,400	46 2 acre lots, 12% imp, H		12% imp, I-	HSG A		
411,475 49		49 5	50-75% Grass cover, Fair, HSG A				
1,2	12,100	30 V	Voods, Go	od, HSG A			
2,1	38,975	38 V	Veighted A	verage			
2,0	77,127	9	7.11% Per	vious Area			
	61,848	2	.89% Impe	ervious Area	а		
Tc	Length	Slope	Velocity	Capacity	Description		
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)			
15.1	50	0.0500	0.06		Sheet Flow,		
					Woods: Dense underbrush n= 0.800 P2= 3.12"		
5.4	725	0.2000	2.24		Shallow Concentrated Flow,		
					Woodland Kv= 5.0 fps		
3.0	510	0.0300	2.79		Shallow Concentrated Flow,		
					Unpaved Kv= 16.1 fps		
23.5	1,285	Total					

# **Subcatchment 8S: Pre-Development Tributary Area to DP**



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#### **Summary for Reach DP: Design Point**

[40] Hint: Not Described (Outflow=Inflow)

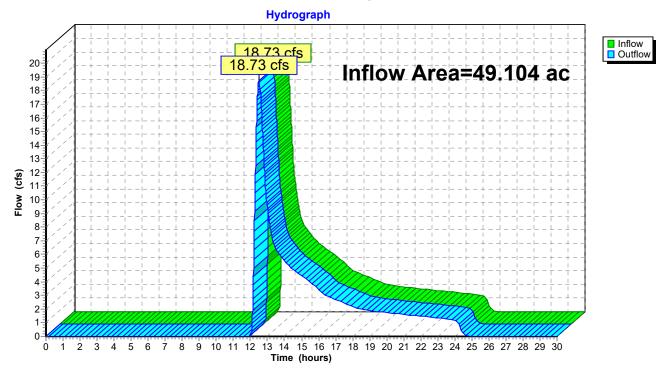
Inflow Area = 49.104 ac, 2.89% Impervious, Inflow Depth = 0.87" for 100-year Storm event

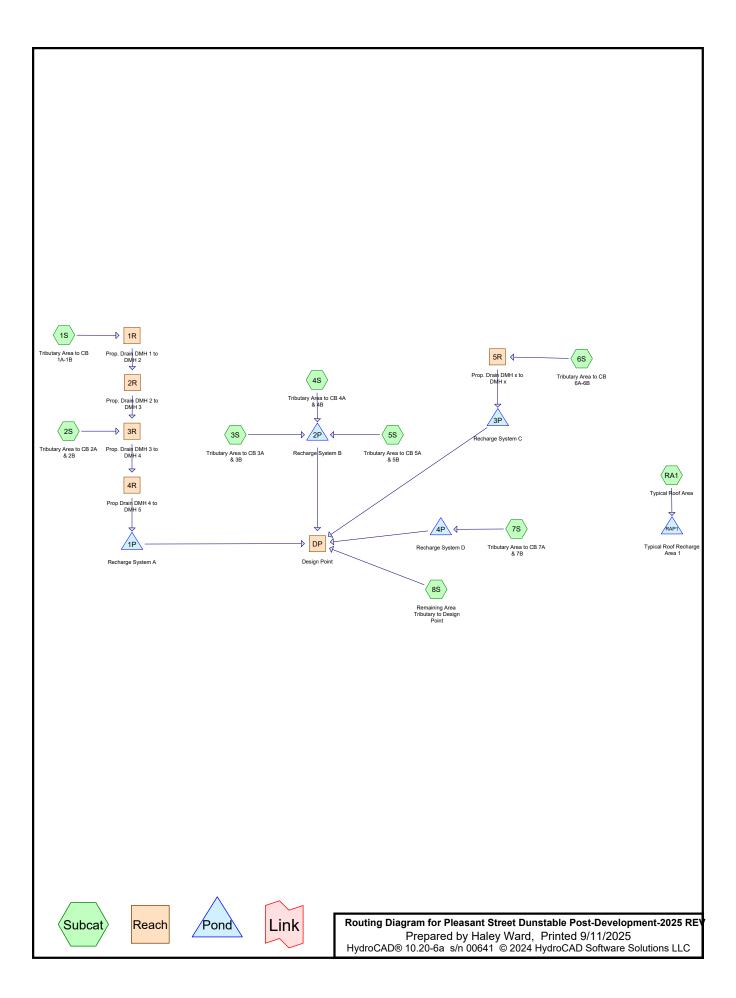
Inflow = 18.73 cfs @ 12.51 hrs, Volume= 3.544 af

Outflow = 18.73 cfs @ 12.51 hrs, Volume= 3.544 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

#### **Reach DP: Design Point**





# Pleasant Street Dunstable Post-Development-2025 REV

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# **Rainfall Events Listing**

<u>.</u>
2
2
2
,

# Pleasant Street Dunstable Post-Development-2025 REV

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# Area Listing (all nodes)

Are	a CN	Description
(acres	s)	(subcatchment-numbers)
13.20	9 46	2 acre lots, 12% imp, HSG A (8S)
2.56	2 49	50-75% Grass cover, Fair, HSG A (8S)
3.21	5 39	>75% Grass cover, Good, HSG A (3S, 4S, 5S, 6S, 7S)
0.24	1 98	Paved parking, HSG A (3S)
1.48	1 98	Paved roads w/curbs & sewers, HSG A (2S, 4S, 5S, 6S, 7S)
0.26	1 98	Roadway, driveways, sidewalks (1S)
0.03	0 98	Roof Area (RA1)
18.99	5 30	Woods, Good, HSG A (8S)
7.95	9 32	Woods/grass comb., Good, HSG A (1S, 2S, 3S, 4S, 5S)
47.95	4 39	TOTAL AREA

# Pleasant Street Dunstable Post-Development-2025 REV

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# Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
47.662	HSG A	1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.291	Other	1S, RA1
47.954		TOTAL AREA

# Pleasant Street Dunstable Post-Development-2025 REV

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# **Ground Covers (all nodes)**

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
	,	, ,	,	, ,	, ,		
13.209	0.000	0.000	0.000	0.000	13.209	2 acre lots, 12% imp	8S
2.562	0.000	0.000	0.000	0.000	2.562	50-75% Grass cover, Fair	8S
3.215	0.000	0.000	0.000	0.000	3.215	>75% Grass cover, Good	3S,
							4S,
							5S,
							6S,
							7S
0.241	0.000	0.000	0.000	0.000	0.241	Paved parking	3S
1.481	0.000	0.000	0.000	0.000	1.481	Paved roads w/curbs & sewers	2S,
							4S,
							5S,
							6S,
							7S
0.000	0.000	0.000	0.000	0.261	0.261	Roadway, driveways, sidewalks	
0.000	0.000	0.000	0.000	0.030	0.030	Roof Area	RA
							1
18.995	0.000	0.000	0.000	0.000	18.995	Woods, Good	8S
7.959	0.000	0.000	0.000	0.000	7.959	Woods/grass comb., Good	1S,
	0.000	0.000	0.000	0.000			2S,
							3S,
							4S,
							5S
47.660	0.000	0.000	0.000	0.204	47 OF 4	TOTAL AREA	55
47.662	0.000	0.000	0.000	0.291	47.954	TOTAL AREA	

# Pleasant Street Dunstable Post-Development-2025 REV

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# Pipe Listing (all nodes)

Line#	Node	In-Invert	Out-Invert	Length	Slope	n	Width	Diam/Height	Inside-Fill	Node
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)	Name
1	1R	168.00	167.10	180.0	0.0050	0.013	0.0	12.0	0.0	
2	2R	167.00	166.10	180.0	0.0050	0.013	0.0	12.0	0.0	
3	3R	166.00	165.45	110.0	0.0050	0.013	0.0	12.0	0.0	
4	4R	165.35	165.21	28.0	0.0050	0.013	0.0	12.0	0.0	
5	5R	164.40	164.00	40.0	0.0100	0.013	0.0	12.0	0.0	

# Pleasant Street Dunstable Post-Development-20Type III 24-hr 2-year Storm Rainfall=3.12" Prepared by Haley Ward Printed 9/11/2025

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

- Subcatchment 1S: Tributary Area to CB Runoff Area=123,030 sf 9.26% Impervious Runoff Depth=0.00" Flow Length=230' Slope=0.0500 '/' Tc=6.0 min CN=38 Runoff=0.00 cfs 0.000 af
- Subcatchment 2S: Tributary Area to CB 2A Runoff Area=35,455 sf 10.14% Impervious Runoff Depth=0.00" Flow Length=120' Slope=0.0200 '/' Tc=6.0 min CN=39 Runoff=0.00 cfs 0.000 af
- Subcatchment 3S: Tributary Area to CB 3A Runoff Area=97,380 sf 10.78% Impervious Runoff Depth=0.00" Flow Length=95' Slope=0.0200 '/' Tc=6.5 min CN=40 Runoff=0.00 cfs 0.000 af
- Subcatchment 4S: Tributary Area to CB 4A Runoff Area=166,140 sf 9.46% Impervious Runoff Depth=0.00" Flow Length=100' Slope=0.0200'/ Tc=6.5 min CN=41 Runoff=0.00 cfs 0.001 af
- Subcatchment 5S: Tributary Area to CB 5A Runoff Area=49,501 sf 8.35% Impervious Runoff Depth=0.00" Flow Length=105' Slope=0.0200 '/' Tc=6.6 min CN=39 Runoff=0.00 cfs 0.000 af
- **Subcatchment 6S: Tributary Area to CB**Flow Length=215'
  Runoff Area=46,170 sf 39.30% Impervious Runoff Depth=0.45"
  Slope=0.0200 '/' Tc=6.8 min CN=62 Runoff=0.35 cfs 0.039 af
- Subcatchment 7S: Tributary Area to CB 7A Runoff Area=55,449 sf 41.36% Impervious Runoff Depth=0.48" Flow Length=266' Slope=0.0200 '/' Tc=7.1 min CN=63 Runoff=0.48 cfs 0.051 af
- **Subcatchment 8S: Remaining Area**Runoff Area=1,514,422 sf 4.56% Impervious Runoff Depth=0.00"
  Flow Length=1,285' Tc=23.5 min CN=37 Runoff=0.00 cfs 0.000 af
- Subcatchment RA1: Typical Roof Area

  Runoff Area=1,310 sf 100.00% Impervious Runoff Depth=2.89"

  Tc=6.0 min CN=98 Runoff=0.09 cfs 0.007 af
- **Reach 1R: Prop. Drain DMH 1 to DMH 2** Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af 12.0" Round Pipe n=0.013 L=180.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=0.00 cfs 0.000 af
- **Reach 2R: Prop. Drain DMH 2 to DMH 3** Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af 12.0" Round Pipe n=0.013 L=180.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=0.00 cfs 0.000 af
- **Reach 3R: Prop. Drain DMH 3 to DMH 4** Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af 12.0" Round Pipe n=0.013 L=110.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=0.00 cfs 0.000 af
- **Reach 4R: Prop Drain DMH 4 to DMH 5** Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af 12.0" Round Pipe n=0.013 L=28.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=0.00 cfs 0.000 af
- **Reach 5R: Prop. Drain DMH x to DMH x** Avg. Flow Depth=0.21' Max Vel=2.88 fps Inflow=0.35 cfs 0.039 af 12.0" Round Pipe n=0.013 L=40.0' S=0.0100 '/' Capacity=3.56 cfs Outflow=0.35 cfs 0.039 af
- Reach DP: Design Point Inflow=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
- Pond 1P: Recharge System A Peak Elev=164.00' Storage=0 cf Inflow=0.00 cfs 0.000 af Discarded=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af

Pleasant Street Dunstable Post-Development-20Type III 24-hr	2-year Storm Rainfall=3.12"
Prepared by Haley Ward	Printed 9/11/2025
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Pond 2P: Recharge System B Peak Elev=162.00' Storage=0 cf Inflow=0.00 cfs 0.001 af Discarded=0.00 cfs 0.001 af Primary=0.00 cfs 0.000 af Outflow=0.00 cfs 0.001 af

Pond 3P: Recharge System C Peak Elev=162.04' Storage=31 cf Inflow=0.35 cfs 0.039 af Discarded=0.33 cfs 0.039 af Primary=0.00 cfs 0.000 af Outflow=0.33 cfs 0.039 af

Pond 4P: Recharge System D Peak Elev=162.04' Storage=42 cf Inflow=0.48 cfs 0.051 af Discarded=0.46 cfs 0.051 af Primary=0.00 cfs 0.000 af Outflow=0.46 cfs 0.051 af

Pond RAP1: Typical Roof Recharge Area 1 Peak Elev=164.43' Storage=32 cf Inflow=0.09 cfs 0.007 af Discarded=0.04 cfs 0.007 af Primary=0.00 cfs 0.000 af Outflow=0.04 cfs 0.007 af

Total Runoff Area = 47.954 ac Runoff Volume = 0.100 af Average Runoff Depth = 0.02" 92.50% Pervious = 44.355 ac 7.50% Impervious = 3.599 ac

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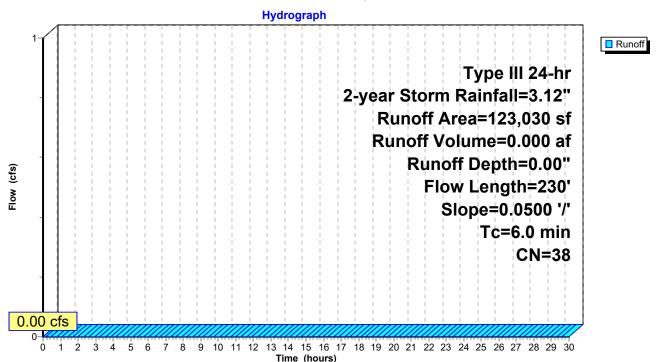
## **Summary for Subcatchment 1S: Tributary Area to CB 1A-1B**

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00" Routed to Reach 1R : Prop. Drain DMH 1 to DMH 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-year Storm Rainfall=3.12"

_	Α	rea (sf)	CN D	escription							
*		11,387									
		11,643				000, N3G A					
		23,030		Veighted A	0						
	1	11,643	9	0.74% Per	vious Area						
11,387 9.26% Impervious Area						a a company of the co					
		,		•							
	Tc	Length	Slope	Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	4.0	50	0.0500	0.21		Sheet Flow,					
						Grass: Short n= 0.150 P2= 3.12"					
	0.7	180	0.0500	4.54		Shallow Concentrated Flow,					
	0.1	100	0.0000	1.01		Paved Kv= 20.3 fps					
_	17	230	Total I	norogod t	o minimum						
	4.7	230	rotal, r	ncreased t	o minimum	Tc = 6.0 min					

# Subcatchment 1S: Tributary Area to CB 1A-1B



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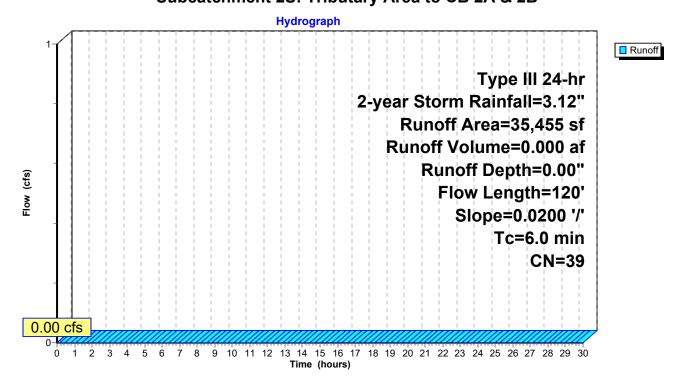
### Summary for Subcatchment 2S: Tributary Area to CB 2A & 2B

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00" Routed to Reach 3R : Prop. Drain DMH 3 to DMH 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-year Storm Rainfall=3.12"

	Α	rea (sf)	CN E	escription					
		3,595	98 F	aved road	s w/curbs 8	k sewers, HSG A			
		31,860	32 V	Voods/gras	s comb., G	Good, HSG A			
		35,455	39 V	Veighted Average					
		31,860	8	9.86% Per	vious Area				
		3,595	1	0.14% Imp	ervious Are	ea			
	Тс	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	0.7	50	0.0200	1.18		Sheet Flow,			
						Smooth surfaces n= 0.011 P2= 3.12"			
	0.4	70	0.0200	2.87		Shallow Concentrated Flow,			
_						Paved Kv= 20.3 fps			
	1.1	120	Total, I	ncreased t	o minimum	Tc = 6.0 min			

# Subcatchment 2S: Tributary Area to CB 2A & 2B



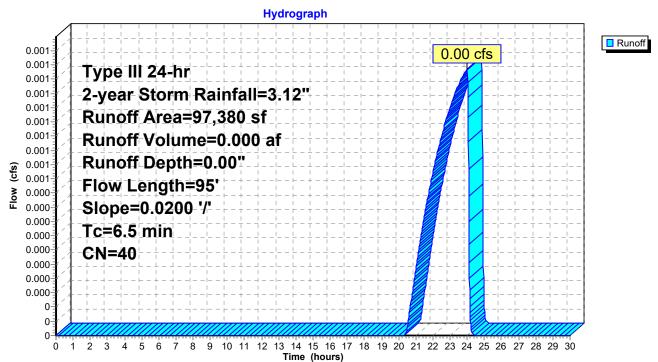
### Summary for Subcatchment 3S: Tributary Area to CB 3A & 3B

Runoff = 0.00 cfs @ 24.00 hrs, Volume= 0.000 af, Depth= 0.00" Routed to Pond 2P : Recharge System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-year Storm Rainfall=3.12"

A	rea (sf)	CN E	escription		
	10,499	98 F	aved park	ing, HSG A	
	15,431	39 >	75% Gras	s cover, Go	ood, HSG A
	71,450	32 V	Voods/gras	ss comb., G	Good, HSG A
	97,380	40 V	Veighted A	verage	
	86,881			vious Area	
	10,499	1	0.78% Imp	ervious Ar	ea
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.7	50	0.0200	0.15		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
0.8	45	0.0200	0.99		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
6.5	95	Total			<u> </u>

# Subcatchment 3S: Tributary Area to CB 3A & 3B



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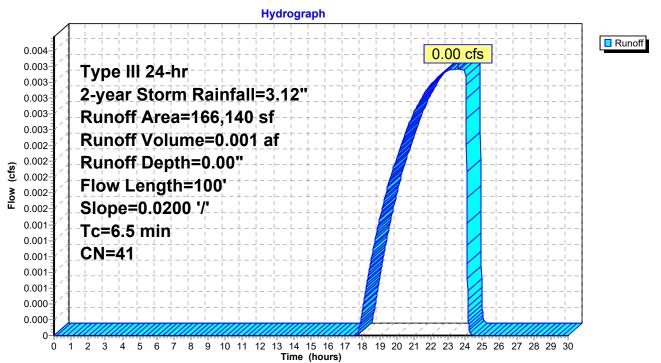
### Summary for Subcatchment 4S: Tributary Area to CB 4A & 4B

Runoff = 0.00 cfs @ 23.58 hrs, Volume= 0.001 af, Depth= 0.00" Routed to Pond 2P : Recharge System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-year Storm Rainfall=3.12"

A	rea (sf)	CN D	escription		
	15,714				& sewers, HSG A
	54,686	39 >	75% Gras	s cover, Go	ood, HSG A
	95,740	32 V	Voods/gras	ss comb., G	Good, HSG A
1	66,140	41 V	Veighted A	verage	
1	50,426	9	0.54% Per	vious Area	
	15,714	9	.46% Impe	ervious Area	a
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.7	50	0.0200	0.15		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
0.8	50	0.0200	0.99		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
6.5	100	Total			<u> </u>

# Subcatchment 4S: Tributary Area to CB 4A & 4B



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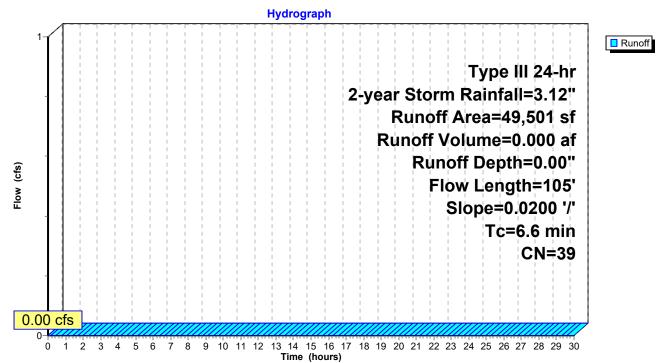
### Summary for Subcatchment 5S: Tributary Area to CB 5A & 5B

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00" Routed to Pond 2P : Recharge System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-year Storm Rainfall=3.12"

A	rea (sf)	CN E	escription		
	4,134				R sewers, HSG A
	9,382	39 >	75% Gras	s cover, Go	ood, HSG A
	35,985	32 V	Voods/gras	ss comb., G	Good, HSG A
	49,501	39 V	Veighted A	verage	
	45,367	9	1.65% Per	vious Area	
	4,134	8	.35% Impe	ervious Area	a
			-		
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.7	50	0.0200	0.15		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
0.9	55	0.0200	0.99		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
6.6	105	Total			<u> </u>

### Subcatchment 5S: Tributary Area to CB 5A & 5B



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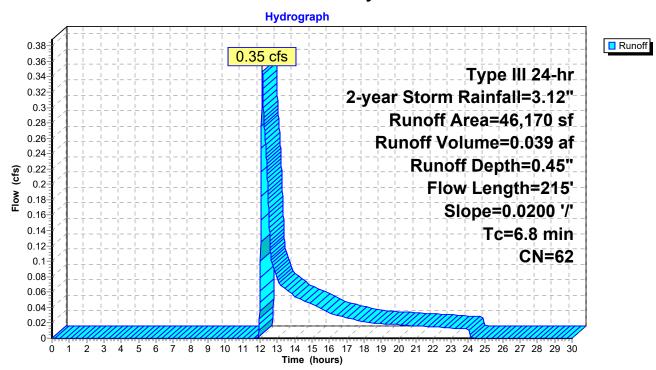
### Summary for Subcatchment 6S: Tributary Area to CB 6A-6B

Runoff = 0.35 cfs @ 12.13 hrs, Volume= 0.039 af, Depth= 0.45" Routed to Reach 5R : Prop. Drain DMH x to DMH x

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-year Storm Rainfall=3.12"

A	rea (sf)	CN E	Description			,		
	18,144		Paved roads w/curbs & sewers, HSG A					
	28,026	39 >	·75% Gras	s cover, Go	ood, HSG A			
	46,170	62 V	Veighted A	verage				
	28,026	6	0.70% Per	vious Area				
	18,144	3	9.30% Imp	ervious Ar	ea			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.7	50	0.0200	0.15		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.12"			
0.6	80	0.0200	2.28		Shallow Concentrated Flow,			
					Unpaved Kv= 16.1 fps			
0.5	85	0.0200	2.87		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
6.8	215	Total						

# Subcatchment 6S: Tributary Area to CB 6A-6B



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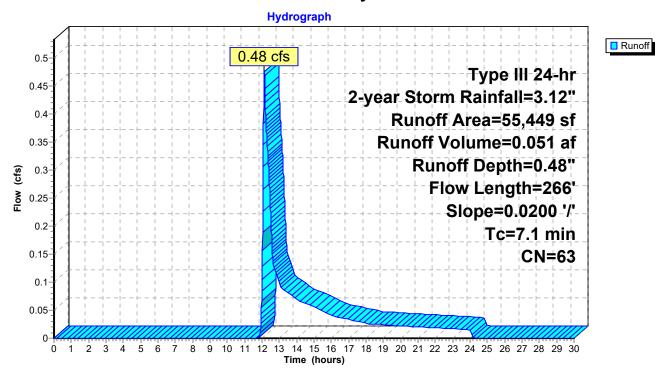
### Summary for Subcatchment 7S: Tributary Area to CB 7A & 7B

Runoff = 0.48 cfs @ 12.13 hrs, Volume= 0.051 af, Depth= 0.48" Routed to Pond 4P : Recharge System D

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-year Storm Rainfall=3.12"

A	rea (sf)	CN E	Description					
	22,933	98 F	Paved roads w/curbs & sewers, HSG A					
	32,516	39 >	75% Gras	s cover, Go	ood, HSG A			
	55,449	63 V	Veighted A	verage				
	32,516	5	8.64% Per	vious Area				
	22,933	4	1.36% Imp	pervious Ar	ea			
_				_				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.7	50	0.0200	0.15		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.12"			
0.5	66	0.0200	2.28		Shallow Concentrated Flow,			
					Unpaved Kv= 16.1 fps			
0.9	150	0.0200	2.87		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
7.1	266	Total						

### Subcatchment 7S: Tributary Area to CB 7A & 7B



### **Summary for Subcatchment 8S: Remaining Area Tributary to Design Point**

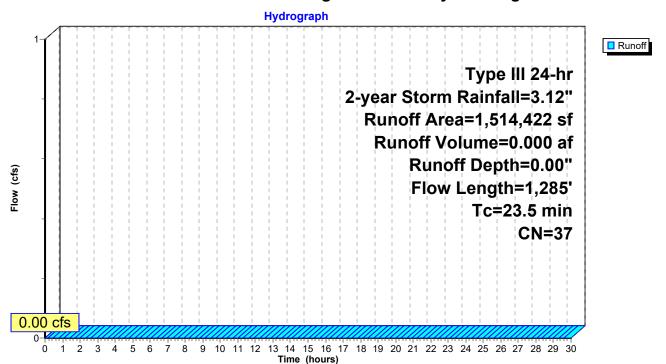
Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Routed to Reach DP: Design Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-year Storm Rainfall=3.12"

	Aı	rea (sf)	CN E	Description						
	5	75,400	46 2	acre lots,	12% imp, I	HSG A				
	1	11,580	49 5	50-75% Grass cover, Fair, HSG A						
	8	27,442	30 V	Noods, Good, HSG A						
	1,5	14,422	37 V	Veighted A	verage					
	1,4	45,374	9	5.44% Per	vious Area					
		69,048	4	.56% Impe	ervious Are	a				
	_									
	Tc	Length	Slope	Velocity	Capacity	Description				
<u>(m</u>	in)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
15	5.1	50	0.0500	0.06		Sheet Flow,				
						Woods: Dense underbrush n= 0.800 P2= 3.12"				
5	5.4	725	0.2000	2.24		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
3	3.0	510	0.0300	2.79		Shallow Concentrated Flow,				
						Unpaved Kv= 16.1 fps				
23	3.5	1,285	Total							

### **Subcatchment 8S: Remaining Area Tributary to Design Point**



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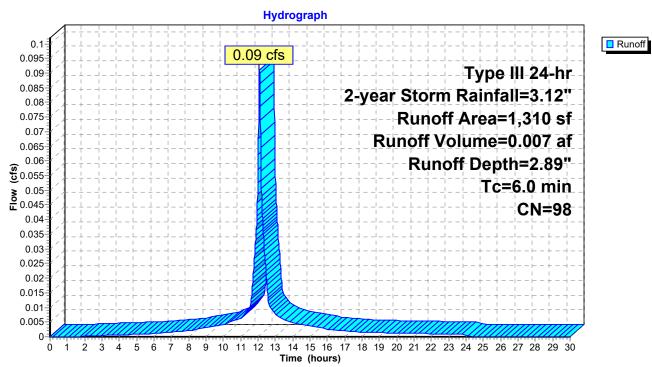
### Summary for Subcatchment RA1: Typical Roof Area

Runoff = 0.09 cfs @ 12.08 hrs, Volume= 0.007 af, Depth= 2.89" Routed to Pond RAP1 : Typical Roof Recharge Area 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-year Storm Rainfall=3.12"

	rea (sf)	CN I	Description		
	1,310	98 I	Roof Area		
	1,310	•	100.00% Im	npervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	·
6.0					Direct Entry,

# **Subcatchment RA1: Typical Roof Area**



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## Summary for Reach 1R: Prop. Drain DMH 1 to DMH 2

Inflow Area = 2.824 ac, 9.26% Impervious, Inflow Depth = 0.00" for 2-year Storm event

Inflow = 0.00 cfs @ 0.00 hrs. Volume = 0.000 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routed to Reach 2R: Prop. Drain DMH 2 to DMH 3

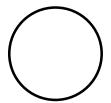
Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

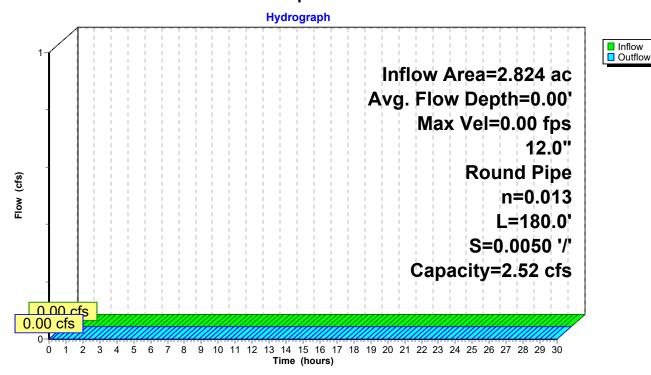
Peak Storage= 0 cf @ 0.00 hrs Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe n= 0.013 Length= 180.0' Slope= 0.0050 '/' Inlet Invert= 168.00', Outlet Invert= 167.10'



#### Reach 1R: Prop. Drain DMH 1 to DMH 2



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### Summary for Reach 2R: Prop. Drain DMH 2 to DMH 3

Inflow Area = 2.824 ac, 9.26% Impervious, Inflow Depth = 0.00" for 2-year Storm event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routed to Reach 3R: Prop. Drain DMH 3 to DMH 4

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min

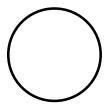
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs

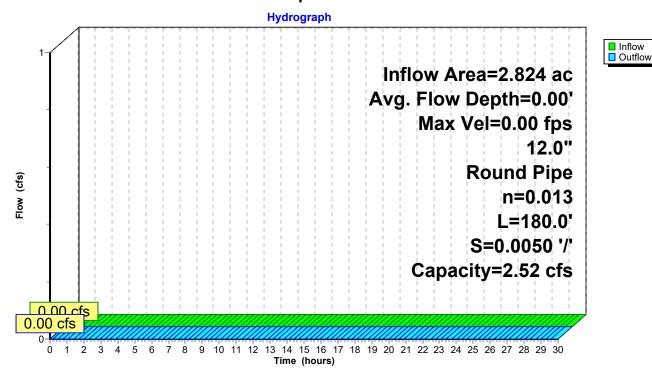
Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe n= 0.013 Length= 180.0' Slope= 0.0050 '/' Inlet Invert= 167.00', Outlet Invert= 166.10'



#### Reach 2R: Prop. Drain DMH 2 to DMH 3



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# Summary for Reach 3R: Prop. Drain DMH 3 to DMH 4

Inflow Area = 3.638 ac, 9.45% Impervious, Inflow Depth = 0.00" for 2-year Storm event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routed to Reach 4R: Prop Drain DMH 4 to DMH 5

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

3 , 111, 3

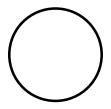
Peak Storage= 0 cf @ 0.00 hrs Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

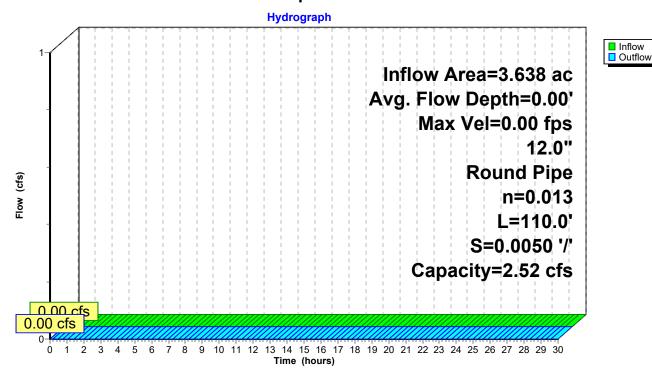
12.0" Round Pipe n= 0.013

Length= 110.0' Slope= 0.0050 '/'

Inlet Invert= 166.00', Outlet Invert= 165.45'



#### Reach 3R: Prop. Drain DMH 3 to DMH 4



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### Summary for Reach 4R: Prop Drain DMH 4 to DMH 5

Inflow Area = 3.638 ac, 9.45% Impervious, Inflow Depth = 0.00" for 2-year Storm event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routed to Pond 1P: Recharge System A

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min

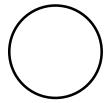
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs

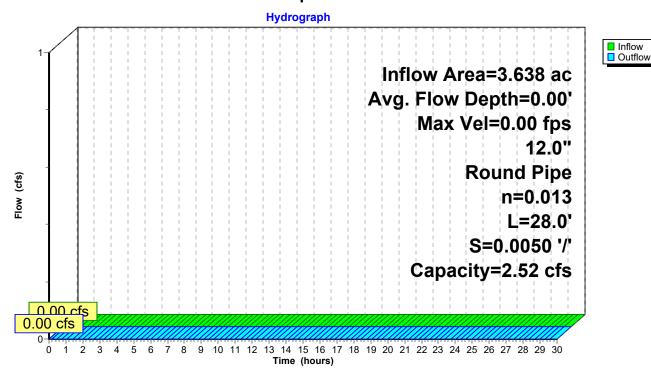
Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 28.0' Slope= 0.0050 '/' Inlet Invert= 165.35', Outlet Invert= 165.21'



#### Reach 4R: Prop Drain DMH 4 to DMH 5



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### Summary for Reach 5R: Prop. Drain DMH x to DMH x

Inflow Area = 1.060 ac, 39.30% Impervious, Inflow Depth = 0.45" for 2-year Storm event

Inflow = 0.35 cfs @ 12.13 hrs, Volume= 0.039 af

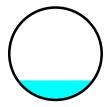
Outflow = 0.35 cfs @ 12.14 hrs, Volume= 0.039 af, Atten= 0%, Lag= 0.4 min

Routed to Pond 3P: Recharge System C

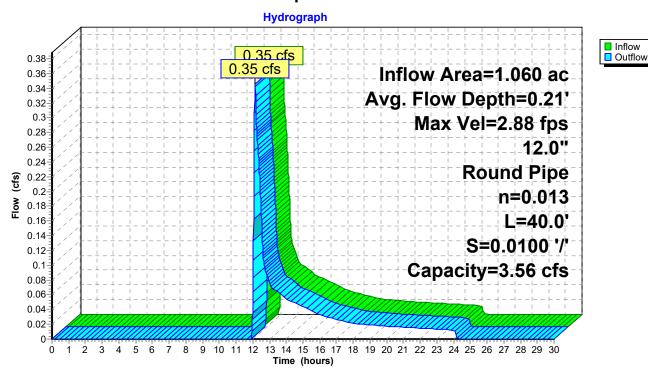
Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 2.88 fps, Min. Travel Time= 0.2 min Avg. Velocity = 1.36 fps, Avg. Travel Time= 0.5 min

Peak Storage= 5 cf @ 12.14 hrs Average Depth at Peak Storage= 0.21', Surface Width= 0.82' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 3.56 cfs

12.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 40.0' Slope= 0.0100 '/' Inlet Invert= 164.40', Outlet Invert= 164.00'



#### Reach 5R: Prop. Drain DMH x to DMH x



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## **Summary for Reach DP: Design Point**

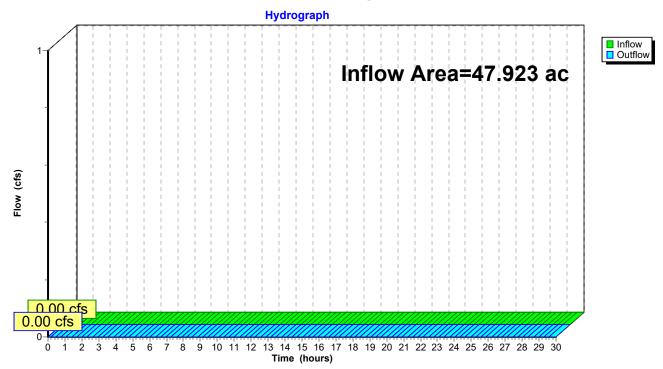
Inflow Area = 47.923 ac, 7.45% Impervious, Inflow Depth = 0.00" for 2-year Storm event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

# **Reach DP: Design Point**



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### Summary for Pond 1P: Recharge System A

Inflow Area = 3.638 ac. 9.45% Impervious, Inflow Depth = 0.00" for 2-year Storm event Inflow 0.00 cfs @ 0.00 hrs. Volume= 0.000 af 0.00 cfs @ 0.00 hrs, Volume= Outflow = 0.000 af, Atten= 0%, Lag= 0.0 min 0.00 hrs, Volume= Discarded = 0.00 cfs @ 0.000 af

Primary 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach DP: Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 164.00' @ 0.00 hrs Surf.Area= 1,309 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert	Avail.Storage	Storage Description
#1	#1 164.50' 1,608		ADS_StormTech SC-740 b +Cap x 35 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	164.00'	1,189 cf	25.25'W x 51.84'L x 3.50'H Prismatoid
			4,581 cf Overall - 1,608 cf Embedded = 2,973 cf x 40.0% Voids
•		2 - 2 - 6	=

2,797 cf Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	164.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 160.00'
#2	Primary	171.25'	6.0" Horiz. Orifice/Grate C= 0.600
	-		I imited to weir flow at low heads

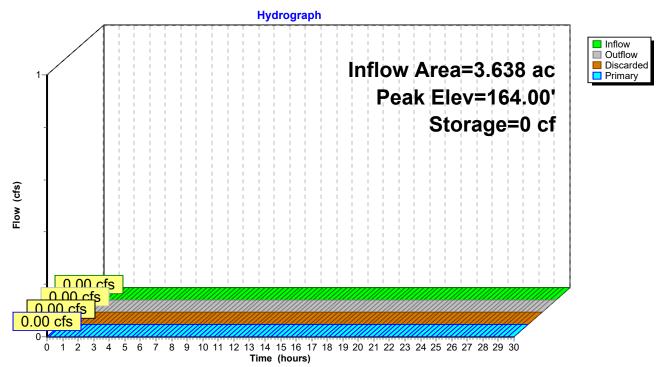
Discarded OutFlow Max=0.00 cfs @ 0.00 hrs HW=164.00' (Free Discharge) 1=Exfiltration (Passes 0.00 cfs of 0.25 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=164.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

AD. Daahanna Oostana A

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## Summary for Pond 2P: Recharge System B

Inflow Area = 7.186 ac, 9.69% Impervious, Inflow Depth = 0.00" for 2-year Storm event 0.00 cfs @ 24.00 hrs, Volume= Inflow 0.001 af

0.00 cfs @ 24.00 hrs, Volume= Outflow = 0.001 af, Atten= 0%, Lag= 0.2 min

0.00 cfs @ 24.00 hrs, Volume= Discarded = 0.001 af Primary 0.000 af 0.00 cfs @ 0.00 hrs, Volume=

Routed to Reach DP: Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 162.00' @ 24.00 hrs Surf.Area= 3,257 sf Storage= 0 cf

Plug-Flow detention time= 1.5 min calculated for 0.001 af (100% of inflow)

Center-of-Mass det. time= 1.5 min (1,307.2 - 1,305.6)

Volume	Invert	Avail.Storage	Storage Description
#1	162.50'	4,135 cf	ADS_StormTech SC-740 b +Cap x 90 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	162.00'	2,907 cf	44.50'W x 73.20'L x 3.50'H Prismatoid
			11,401 cf Overall - 4,135 cf Embedded = 7,266 cf x 40.0% Voids

7,041 cf Total Available Storage

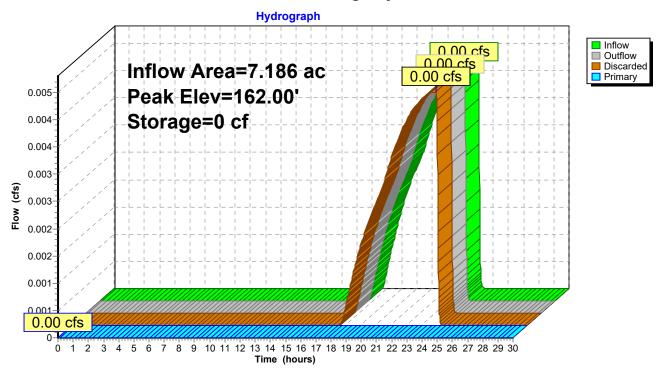
Device	Routing	Invert	Outlet Devices
#1	Discarded	162.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 160.00'
#2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

Discarded OutFlow Max=0.62 cfs @ 24.00 hrs HW=162.00' (Free Discharge) 1=Exfiltration (Controls 0.62 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=162.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

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# Pond 2P: Recharge System B



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### Summary for Pond 3P: Recharge System C

Inflow Area = 1.060 ac, 39.30% Impervious, Inflow Depth = 0.45" for 2-year Storm event Inflow = 0.35 cfs @ 12.14 hrs, Volume= 0.039 af Outflow = 0.33 cfs @ 12.17 hrs, Volume= 0.039 af, Atten= 4%, Lag= 1.7 min Discarded = 0.03 cfs @ 12.17 hrs, Volume= 0.039 af Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach DP : Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 162.04' @ 12.17 hrs Surf.Area= 1,848 sf Storage= 31 cf

Plug-Flow detention time= 1.5 min calculated for 0.039 af (100% of inflow)

Center-of-Mass det. time= 1.5 min ( 915.5 - 914.0 )

Volume	Invert	Avail.Storage	Storage Description
#1	162.50'	2,297 cf	ADS_StormTech SC-740 b +Cap x 50 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	162.00'	1,669 cf	25.25'W x 73.20'L x 3.50'H Prismatoid
			6,469 cf Overall - 2,297 cf Embedded = 4,172 cf x 40.0% Voids
	<u> </u>		

3,966 cf Total Available Storage

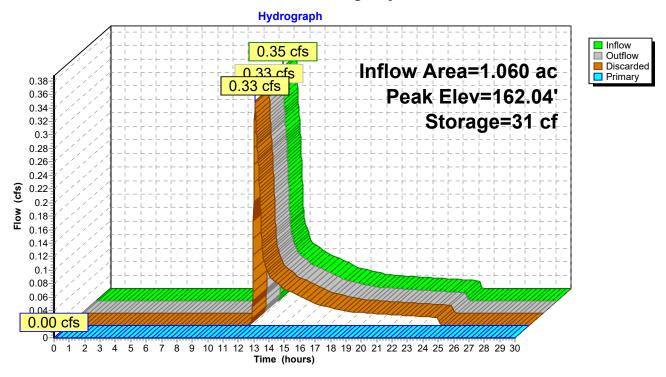
Device	Routing	Invert	Outlet Devices
#1	Discarded	162.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 160.00'
#2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

Discarded OutFlow Max=0.36 cfs @ 12.17 hrs HW=162.04' (Free Discharge) 1=Exfiltration (Controls 0.36 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=162.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

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# Pond 3P: Recharge System C



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### Summary for Pond 4P: Recharge System D

Inflow Area = 1.273 ac, 41.36% Impervious, Inflow Depth = 0.48" for 2-year Storm event

Inflow = 0.48 cfs @ 12.13 hrs, Volume= 0.051 af

Outflow = 0.46 cfs @ 12.16 hrs, Volume= 0.051 af, Atten= 4%, Lag= 1.7 min

Discarded = 0.46 cfs @ 12.16 hrs, Volume= 0.051 af Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach DP : Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 162.04' @ 12.16 hrs Surf.Area= 2,544 sf Storage= 42 cf

Plug-Flow detention time= 1.5 min calculated for 0.051 af (100% of inflow)

Center-of-Mass det. time= 1.5 min ( 910.0 - 908.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	162.50'	3,216 cf	ADS_StormTech SC-740 b +Cap x 70 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	162.00'	2,275 cf	34.75'W x 73.20'L x 3.50'H Prismatoid
			8,903 cf Overall - 3,216 cf Embedded = 5,687 cf x 40.0% Voids

5,491 cf Total Available Storage

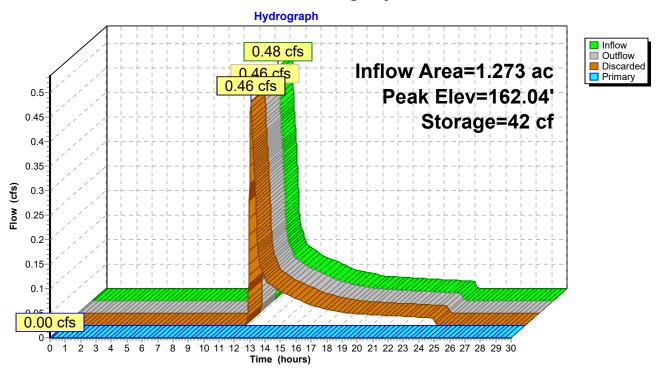
Device	Routing	Invert	Outlet Devices
#1	Discarded	162.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 160.00'
#2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

**Discarded OutFlow** Max=0.50 cfs @ 12.16 hrs HW=162.04' (Free Discharge) 1=Exfiltration (Controls 0.50 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=162.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

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# Pond 4P: Recharge System D



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# Summary for Pond RAP1: Typical Roof Recharge Area 1

Inflow Area = 0.030 ac,100.00% Impervious, Inflow Depth = 2.89" for 2-year Storm event 
Inflow = 0.09 cfs @ 12.08 hrs, Volume= 0.007 af 
Outflow = 0.04 cfs @ 12.27 hrs, Volume= 0.007 af, Atten= 57%, Lag= 10.9 min 
Discarded = 0.00 cfs @ 12.27 hrs, Volume= 0.007 af 
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 164.43' @ 12.27 hrs Surf.Area= 187 sf Storage= 32 cf

Plug-Flow detention time= 3.6 min calculated for 0.007 af (100% of inflow) Center-of-Mass det. time= 3.6 min (760.5 - 756.9)

Volume	Invert	Avail.Storage	Storage Description
#1	1 164.50'		ADS_StormTech SC-310 x 6 Inside #2
			Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf
			Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap
#2	164.00'	139 cf	11.50'W x 16.24'L x 2.33'H Prismatoid
			435 cf Overall - 88 cf Embedded = 347 cf x 40.0% Voids
		227 cf	Total Available Storage

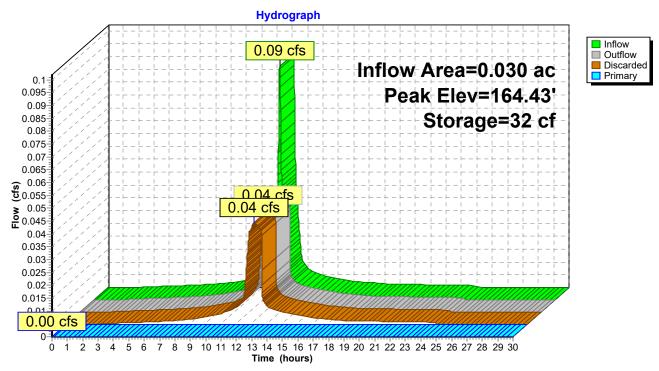
Device	Routing	Invert	Outlet Devices
#1	Discarded	164.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 160.00'
#2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

Discarded OutFlow Max=0.04 cfs @ 12.27 hrs HW=164.43' (Free Discharge) 1=Exfiltration (Controls 0.04 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=164.00' (Free Discharge) 2=Orifice/Grate (Controls 0.00 cfs)

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# Pond RAP1: Typical Roof Recharge Area 1



# Pleasant Street Dunstable Post-Development-2Type III 24-hr 10-year Storm Rainfall=4.81" Prepared by Haley Ward Printed 9/11/2025

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

- Subcatchment 1S: Tributary Area to CB Runoff Area=123,030 sf 9.26% Impervious Runoff Depth=0.13" Flow Length=230' Slope=0.0500 '/' Tc=6.0 min CN=38 Runoff=0.05 cfs 0.032 af
- Subcatchment 2S: Tributary Area to CB 2A Runoff Area=35,455 sf 10.14% Impervious Runoff Depth=0.16" Flow Length=120' Slope=0.0200 '/' Tc=6.0 min CN=39 Runoff=0.02 cfs 0.011 af
- Subcatchment 3S: Tributary Area to CB 3A Runoff Area=97,380 sf 10.78% Impervious Runoff Depth=0.19" Flow Length=95' Slope=0.0200 '/' Tc=6.5 min CN=40 Runoff=0.08 cfs 0.036 af
- Subcatchment 4S: Tributary Area to CB 4A Runoff Area=166,140 sf 9.46% Impervious Runoff Depth=0.23" Flow Length=100' Slope=0.0200 '/' Tc=6.5 min CN=41 Runoff=0.22 cfs 0.073 af
- Subcatchment 5S: Tributary Area to CB 5A Runoff Area=49,501 sf 8.35% Impervious Runoff Depth=0.16" Flow Length=105' Slope=0.0200'/' Tc=6.6 min CN=39 Runoff=0.03 cfs 0.015 af
- Subcatchment 6S: Tributary Area to CB Runoff Area=46,170 sf 39.30% Impervious Runoff Depth=1.32" Flow Length=215' Slope=0.0200 '/' Tc=6.8 min CN=62 Runoff=1.45 cfs 0.117 af
- Subcatchment 7S: Tributary Area to CB 7A Runoff Area=55,449 sf 41.36% Impervious Runoff Depth=1.39" Flow Length=266' Slope=0.0200 '/' Tc=7.1 min CN=63 Runoff=1.84 cfs 0.147 af
- **Subcatchment 8S: Remaining Area**Runoff Area=1,514,422 sf 4.56% Impervious Runoff Depth=0.11"

  Flow Length=1,285' Tc=23.5 min CN=37 Runoff=0.49 cfs 0.310 af
- Subcatchment RA1: Typical Roof Area

  Runoff Area=1,310 sf 100.00% Impervious Runoff Depth=4.57"

  Tc=6.0 min CN=98 Runoff=0.14 cfs 0.011 af
- **Reach 1R: Prop. Drain DMH 1 to DMH 2** Avg. Flow Depth=0.10' Max Vel=1.27 fps Inflow=0.05 cfs 0.032 af 12.0" Round Pipe n=0.013 L=180.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=0.05 cfs 0.032 af
- **Reach 2R: Prop. Drain DMH 2 to DMH 3** Avg. Flow Depth=0.10' Max Vel=1.27 fps Inflow=0.05 cfs 0.032 af 12.0" Round Pipe n=0.013 L=180.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=0.05 cfs 0.032 af
- **Reach 3R: Prop. Drain DMH 3 to DMH 4** Avg. Flow Depth=0.11' Max Vel=1.40 fps Inflow=0.07 cfs 0.043 af 12.0" Round Pipe n=0.013 L=110.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=0.07 cfs 0.043 af
- **Reach 4R: Prop Drain DMH 4 to DMH 5** Avg. Flow Depth=0.11' Max Vel=1.40 fps Inflow=0.07 cfs 0.043 af 12.0" Round Pipe n=0.013 L=28.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=0.07 cfs 0.043 af
- **Reach 5R: Prop. Drain DMH x to DMH x** Avg. Flow Depth=0.44' Max Vel=4.31 fps Inflow=1.45 cfs 0.117 af 12.0" Round Pipe n=0.013 L=40.0' S=0.0100 '/' Capacity=3.56 cfs Outflow=1.45 cfs 0.117 af
- Reach DP: Design Point Inflow=0.49 cfs 0.310 af
  Outflow=0.49 cfs 0.310 af
- Pond 1P: Recharge System A Peak Elev=164.02' Storage=10 cf Inflow=0.07 cfs 0.043 af Discarded=0.07 cfs 0.043 af Primary=0.00 cfs 0.000 af Outflow=0.07 cfs 0.043 af

Pleasant Street Dunstable Post-Development-2Type III 24-hr 10-year Storm Rainfall=4.81"
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Pond 2P: Recharge System B Peak Elev=162.02' Storage=29 cf Inflow=0.32 cfs 0.124 af Discarded=0.31 cfs 0.124 af Primary=0.00 cfs 0.000 af Outflow=0.31 cfs 0.124 af

Pond 3P: Recharge System C Peak Elev=162.83' Storage=877 cf Inflow=1.45 cfs 0.117 af Discarded=0.50 cfs 0.117 af Primary=0.00 cfs 0.000 af Outflow=0.50 cfs 0.117 af

Pond 4P: Recharge System D Peak Elev=162.76' Storage=1,050 cf Inflow=1.84 cfs 0.147 af Discarded=0.67 cfs 0.147 af Primary=0.00 cfs 0.000 af Outflow=0.67 cfs 0.147 af

Pond RAP1: Typical Roof Recharge Area 1 Peak Elev=164.84' Storage=82 cf Inflow=0.14 cfs 0.011 af Discarded=0.04 cfs 0.011 af Primary=0.00 cfs 0.000 af Outflow=0.04 cfs 0.011 af

Total Runoff Area = 47.954 ac Runoff Volume = 0.753 af Average Runoff Depth = 0.19" 92.50% Pervious = 44.355 ac 7.50% Impervious = 3.599 ac

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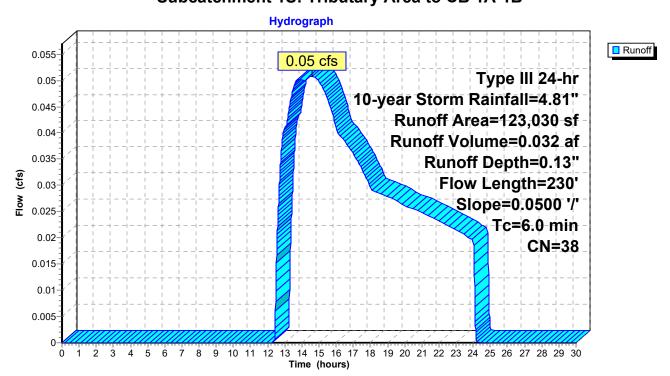
# Summary for Subcatchment 1S: Tributary Area to CB 1A-1B

Runoff = 0.05 cfs @ 14.58 hrs, Volume= 0.032 af, Depth= 0.13" Routed to Reach 1R : Prop. Drain DMH 1 to DMH 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Storm Rainfall=4.81"

	Α	rea (sf)	CN D	escription			
*		11,387	98 R	Roadway, driveways, sidewalks			
_	1	11,643	32 V	Voods/gras	ss comb., G	Good, HSG A	
123,030 38 Weighted Average					verage		
	1	11,643	9	0.74% Per	vious Area		
11,387 9.26% Impervious Area						a	
	_				_		
	Tc	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	4.0	50	0.0500	0.21		Sheet Flow,	
						Grass: Short n= 0.150 P2= 3.12"	
	0.7	180	0.0500	4.54		Shallow Concentrated Flow,	
_						Paved Kv= 20.3 fps	
	4.7	230	Total, I	ncreased t	o minimum	Tc = 6.0 min	

# Subcatchment 1S: Tributary Area to CB 1A-1B



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# Summary for Subcatchment 2S: Tributary Area to CB 2A & 2B

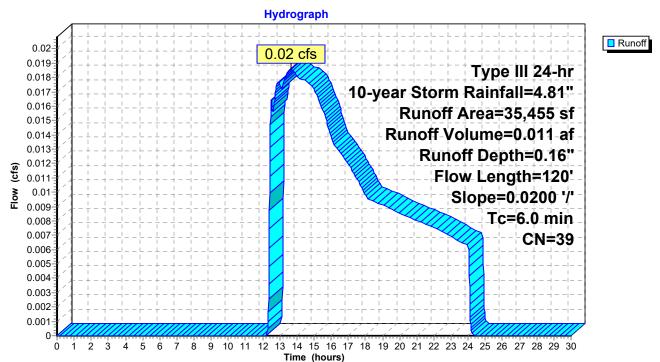
Runoff = 0.02 cfs @ 13.66 hrs, Volume= 0.011 af, Depth= 0.16" Routed to Reach 3R : Prop. Drain DMH 3 to DMH 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Storm Rainfall=4.81"

	Α	rea (sf)	CN E	N Description				
		3,595	98 F	aved road	s w/curbs 8	& sewers, HSG A		
		31,860	32 V	Voods/gras	s comb., G	Good, HSG A		
		35,455	39 V	Veighted A	verage			
	31,860 89.86% Pervious Area							
		3,595	10.14% Impervious Area					
	Тс	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	0.7	50	0.0200	1.18		Sheet Flow,		
						Smooth surfaces n= 0.011 P2= 3.12"		
	0.4	70	0.0200	2.87		Shallow Concentrated Flow,		
_						Paved Kv= 20.3 fps		
	4 4	400	Tatal I	4		To = 6.0 min		

1.1 120 Total, Increased to minimum Tc = 6.0 min

# Subcatchment 2S: Tributary Area to CB 2A & 2B



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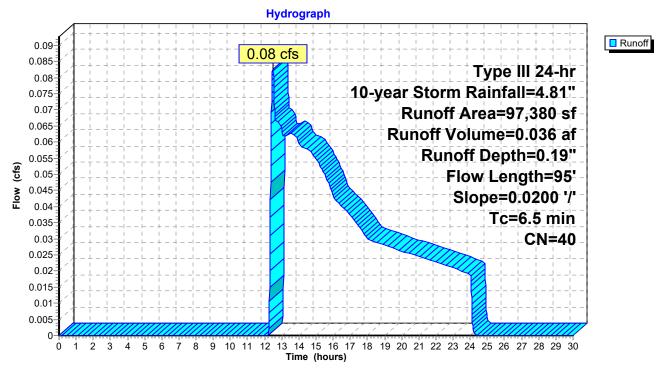
### Summary for Subcatchment 3S: Tributary Area to CB 3A & 3B

Runoff = 0.08 cfs @ 12.48 hrs, Volume= 0.036 af, Depth= 0.19" Routed to Pond 2P : Recharge System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Storm Rainfall=4.81"

A	rea (sf)	CN [	Description					
	10,499	98 F	Paved parking, HSG A					
	15,431	39 >	>75% Grass cover, Good, HSG A					
	71,450	32 V	Woods/grass comb., Good, HSG A					
	97,380	40 Weighted Average						
	86,881	89.22% Pervious Area						
10,499 10.78% Impervious Are			0.78% Imp	pervious Ar	ea			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·			
5.7	50	0.0200	0.15		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.12"			
8.0	45	0.0200	0.99		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
6.5	95	Total			·			

# Subcatchment 3S: Tributary Area to CB 3A & 3B



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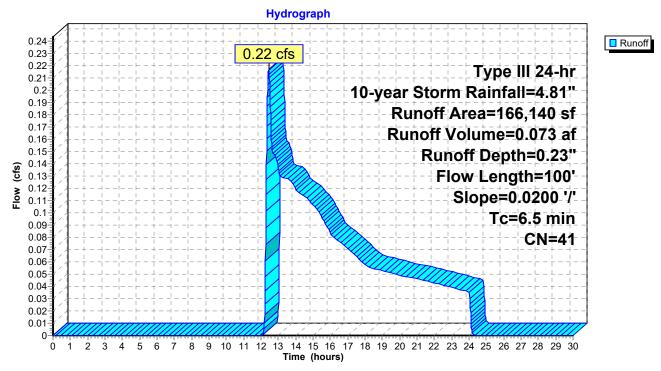
### Summary for Subcatchment 4S: Tributary Area to CB 4A & 4B

Runoff = 0.22 cfs @ 12.44 hrs, Volume= 0.073 af, Depth= 0.23" Routed to Pond 2P : Recharge System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Storm Rainfall=4.81"

	Area (sf)	CN Description						
	15,714	98 Paved roads w/curbs & sewers, HSG A						
	54,686	39 >	39 >75% Grass cover, Good, HSG A					
	95,740	32 Woods/grass comb., Good, HSG A						
	166,140 41 Weighted Average							
150,426		90.54% Pervious Area						
15,714		9.46% Impervious Area						
			-					
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·			
5.7	50	0.0200	0.15		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.12"			
0.8	50	0.0200	0.99		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
6.5	100	Total			<u> </u>			

## Subcatchment 4S: Tributary Area to CB 4A & 4B



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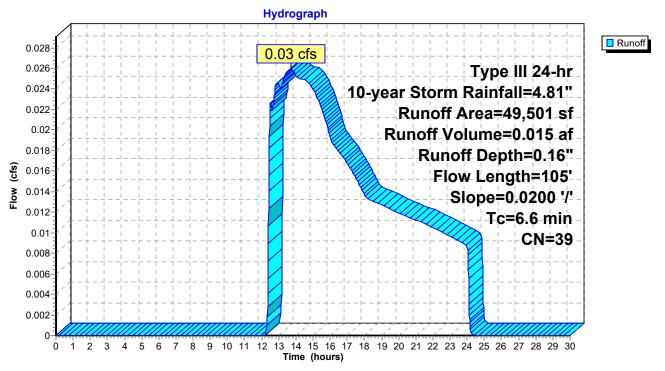
### Summary for Subcatchment 5S: Tributary Area to CB 5A & 5B

Runoff = 0.03 cfs @ 13.69 hrs, Volume= 0.015 af, Depth= 0.16" Routed to Pond 2P : Recharge System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Storm Rainfall=4.81"

A	rea (sf)	CN Description						
	4,134	98 Paved roads w/curbs & sewers, HSG A						
	9,382	39 >	39 >75% Grass cover, Good, HSG A					
	35,985	32 V	32 Woods/grass comb., Good, HSG A					
	49,501	39 Weighted Average						
	45,367	15,367 91.65% Pervious Area						
	4,134	8	.35% Impe	a				
			-					
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.7	50	0.0200	0.15		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.12"			
0.9	55	0.0200	0.99		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
6.6	105	Total						

# Subcatchment 5S: Tributary Area to CB 5A & 5B



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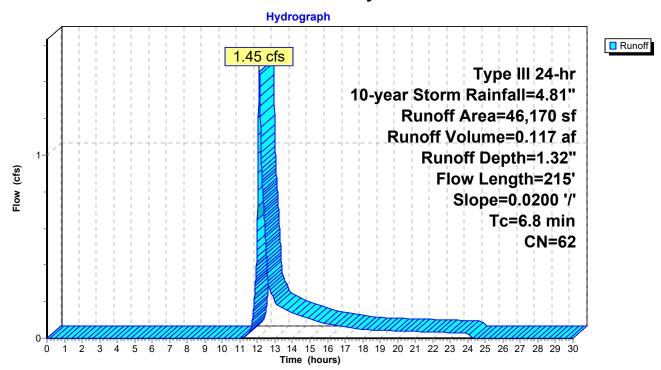
## Summary for Subcatchment 6S: Tributary Area to CB 6A-6B

Runoff = 1.45 cfs @ 12.11 hrs, Volume= 0.117 af, Depth= 1.32" Routed to Reach 5R : Prop. Drain DMH x to DMH x

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Storm Rainfall=4.81"

A	rea (sf)	CN E	Description			,	
	18,144						
	28,026	39 >	·75% Gras	s cover, Go	ood, HSG A		
	46,170	62 V	Veighted A	verage			
	28,026	6	0.70% Per	vious Area			
	18,144	3	9.30% Imp	ervious Ar	ea		
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
5.7	50	0.0200	0.15		Sheet Flow,		
					Grass: Short n= 0.150 P2= 3.12"		
0.6	80	0.0200	2.28		Shallow Concentrated Flow,		
					Unpaved Kv= 16.1 fps		
0.5	85	0.0200	2.87		Shallow Concentrated Flow,		
					Paved Kv= 20.3 fps		
6.8	215	Total					

## Subcatchment 6S: Tributary Area to CB 6A-6B



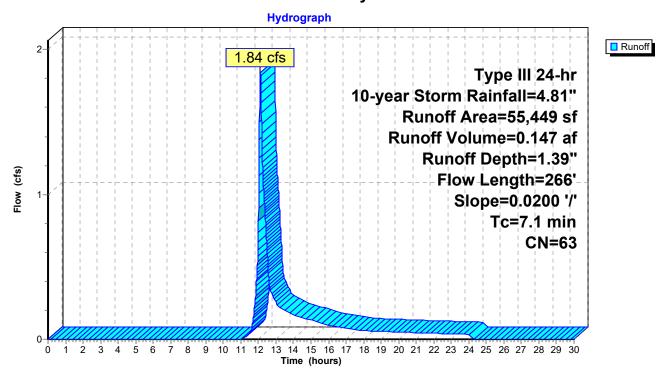
## Summary for Subcatchment 7S: Tributary Area to CB 7A & 7B

Runoff = 1.84 cfs @ 12.11 hrs, Volume= 0.147 af, Depth= 1.39" Routed to Pond 4P : Recharge System D

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Storm Rainfall=4.81"

/	Area (sf)	CN E	escription					
	22,933	98 F	98 Paved roads w/curbs & sewers, HSG A					
	32,516	39 >	75% Gras	s cover, Go	ood, HSG A			
	55,449	63 V	Veighted A	verage				
	32,516	5	8.64% Per	vious Area				
	22,933	4	1.36% Imp	ervious Ar	ea			
_								
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.7	50	0.0200	0.15		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.12"			
0.5	66	0.0200	2.28		Shallow Concentrated Flow,			
					Unpaved Kv= 16.1 fps			
0.9	150	0.0200	2.87		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
7.1	266	Total						

## Subcatchment 7S: Tributary Area to CB 7A & 7B



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## **Summary for Subcatchment 8S: Remaining Area Tributary to Design Point**

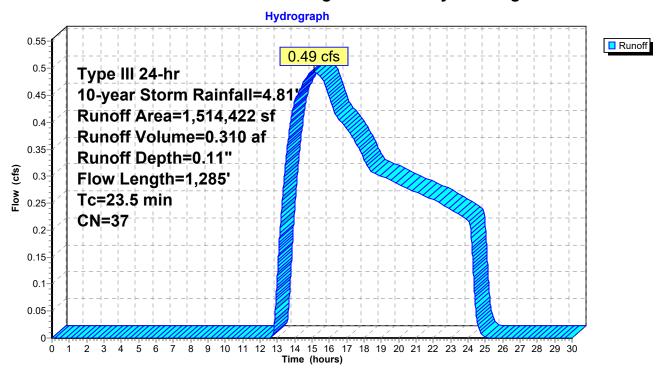
Runoff = 0.49 cfs @ 15.12 hrs, Volume= 0.310 af, Depth= 0.11"

Routed to Reach DP: Design Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Storm Rainfall=4.81"

A	rea (sf)	CN E	escription		
5	75,400	46 2	acre lots,	12% imp, F	HSG A
1	11,580	49 5	0-75% Gra	ass cover, F	Fair, HSG A
8	27,442	30 V	Voods, Go	od, HSG A	
1,5	14,422	37 V	Veighted A	verage	
1,4	45,374	9	5.44% Per	vious Area	
	69,048	4	.56% Impe	ervious Area	a
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
15.1	50	0.0500	0.06		Sheet Flow,
					Woods: Dense underbrush n= 0.800 P2= 3.12"
5.4	725	0.2000	2.24		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
3.0	510	0.0300	2.79		Shallow Concentrated Flow,
					Unpaved Kv= 16.1 fps
23.5	1,285	Total			

#### **Subcatchment 8S: Remaining Area Tributary to Design Point**



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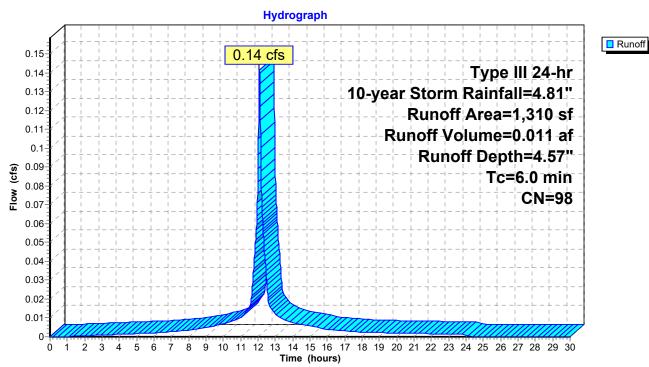
## **Summary for Subcatchment RA1: Typical Roof Area**

Runoff = 0.14 cfs @ 12.08 hrs, Volume= 0.011 af, Depth= 4.57" Routed to Pond RAP1 : Typical Roof Recharge Area 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Storm Rainfall=4.81"

	Α	rea (sf)	CN [	Description		
		1,310	98 F	Roof Area		
		1,310	•	100.00% Im	pervious A	Area
	Тс	Length	Slope	Velocity	Canacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Beschption
-	6.0					Direct Entry

## **Subcatchment RA1: Typical Roof Area**



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## Summary for Reach 1R: Prop. Drain DMH 1 to DMH 2

Inflow Area = 2.824 ac, 9.26% Impervious, Inflow Depth = 0.13" for 10-year Storm event

Inflow = 0.05 cfs @ 14.58 hrs, Volume= 0.032 af

Outflow = 0.05 cfs @ 14.63 hrs, Volume= 0.032 af, Atten= 0%, Lag= 3.0 min

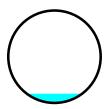
Routed to Reach 2R: Prop. Drain DMH 2 to DMH 3

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 1.27 fps, Min. Travel Time= 2.4 min

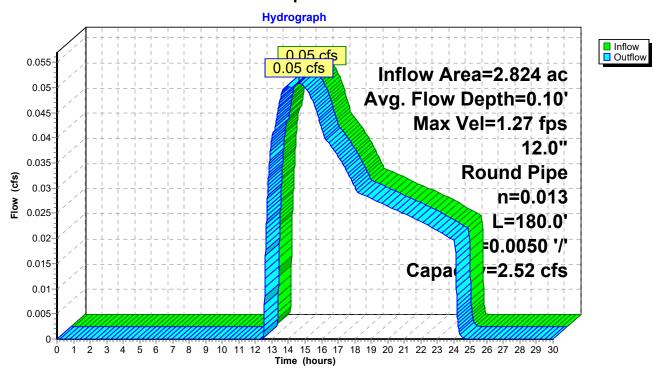
Avg. Velocity = 1.06 fps, Avg. Travel Time= 2.8 min

Peak Storage= 7 cf @ 14.59 hrs Average Depth at Peak Storage= 0.10', Surface Width= 0.60' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe n= 0.013 Length= 180.0' Slope= 0.0050 '/' Inlet Invert= 168.00', Outlet Invert= 167.10'



#### Reach 1R: Prop. Drain DMH 1 to DMH 2



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## Summary for Reach 2R: Prop. Drain DMH 2 to DMH 3

Inflow Area = 2.824 ac, 9.26% Impervious, Inflow Depth = 0.13" for 10-year Storm event

Inflow = 0.05 cfs @ 14.63 hrs, Volume= 0.032 af

Outflow = 0.05 cfs @ 14.70 hrs, Volume= 0.032 af, Atten= 0%, Lag= 4.3 min

Routed to Reach 3R: Prop. Drain DMH 3 to DMH 4

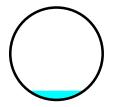
Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Max. Velocity= 1.27 fps, Min. Travel Time= 2.4 min Avg. Velocity = 1.04 fps, Avg. Travel Time= 2.9 min

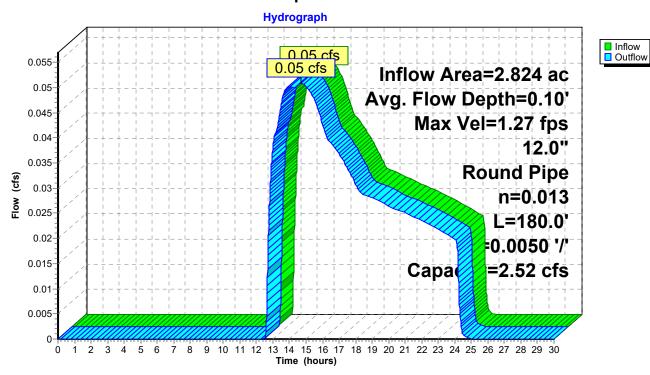
Peak Storage= 7 cf @ 14.66 hrs

Average Depth at Peak Storage= 0.10', Surface Width= 0.60' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe n= 0.013 Length= 180.0' Slope= 0.0050 '/' Inlet Invert= 167.00', Outlet Invert= 166.10'



#### Reach 2R: Prop. Drain DMH 2 to DMH 3



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## Summary for Reach 3R: Prop. Drain DMH 3 to DMH 4

Inflow Area = 3.638 ac, 9.45% Impervious, Inflow Depth = 0.14" for 10-year Storm event

Inflow = 0.07 cfs @ 13.90 hrs, Volume= 0.043 af

Outflow = 0.07 cfs @ 13.94 hrs, Volume= 0.043 af, Atten= 0%, Lag= 2.4 min

Routed to Reach 4R: Prop Drain DMH 4 to DMH 5

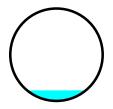
Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 1.40 fps, Min. Travel Time= 1.3 min

Avg. Velocity = 1.12 fps, Avg. Travel Time= 1.5 min

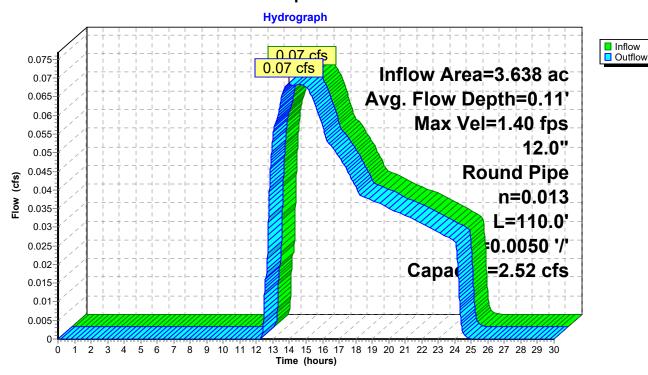
Peak Storage= 5 cf @ 13.92 hrs

Average Depth at Peak Storage= 0.11', Surface Width= 0.63' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe n= 0.013 Length= 110.0' Slope= 0.0050 '/' Inlet Invert= 166.00', Outlet Invert= 165.45'



#### Reach 3R: Prop. Drain DMH 3 to DMH 4



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#### Summary for Reach 4R: Prop Drain DMH 4 to DMH 5

Inflow Area = 3.638 ac, 9.45% Impervious, Inflow Depth = 0.14" for 10-year Storm event

Inflow = 0.07 cfs @ 13.94 hrs, Volume= 0.043 af

Outflow = 0.07 cfs @ 13.95 hrs, Volume= 0.043 af, Atten= 0%, Lag= 0.6 min

Routed to Pond 1P: Recharge System A

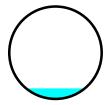
Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 1.40 fps, Min. Travel Time= 0.3 min

Avg. Velocity = 1.12 fps, Avg. Travel Time= 0.4 min

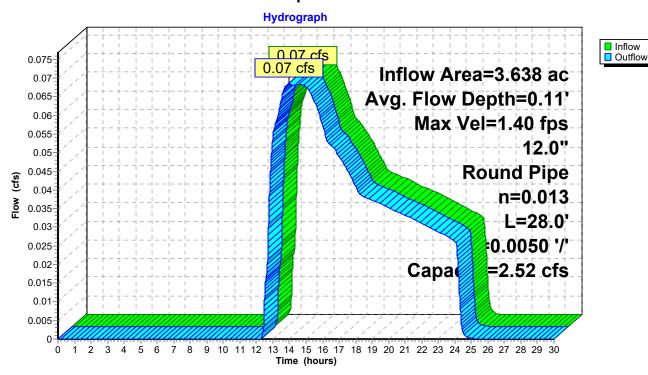
Peak Storage= 1 cf @ 13.95 hrs

Average Depth at Peak Storage= 0.11', Surface Width= 0.63' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 28.0' Slope= 0.0050 '/' Inlet Invert= 165.35', Outlet Invert= 165.21'



#### Reach 4R: Prop Drain DMH 4 to DMH 5



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#### Summary for Reach 5R: Prop. Drain DMH x to DMH x

Inflow Area = 1.060 ac, 39.30% Impervious, Inflow Depth = 1.32" for 10-year Storm event

Inflow = 1.45 cfs @ 12.11 hrs, Volume= 0.117 af

Outflow = 1.45 cfs @ 12.11 hrs, Volume= 0.117 af, Atten= 0%, Lag= 0.3 min

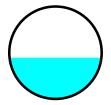
Routed to Pond 3P: Recharge System C

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 4.31 fps, Min. Travel Time= 0.2 min

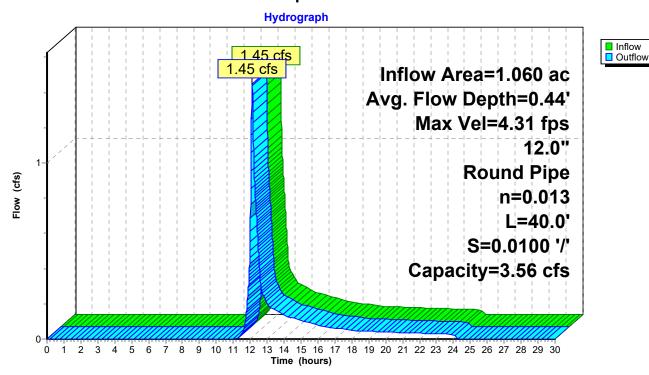
Avg. Velocity = 1.78 fps, Avg. Travel Time= 0.4 min

Peak Storage= 14 cf @ 12.11 hrs Average Depth at Peak Storage= 0.44', Surface Width= 0.99' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 3.56 cfs

12.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 40.0' Slope= 0.0100 '/' Inlet Invert= 164.40', Outlet Invert= 164.00'



#### Reach 5R: Prop. Drain DMH x to DMH x



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# **Summary for Reach DP: Design Point**

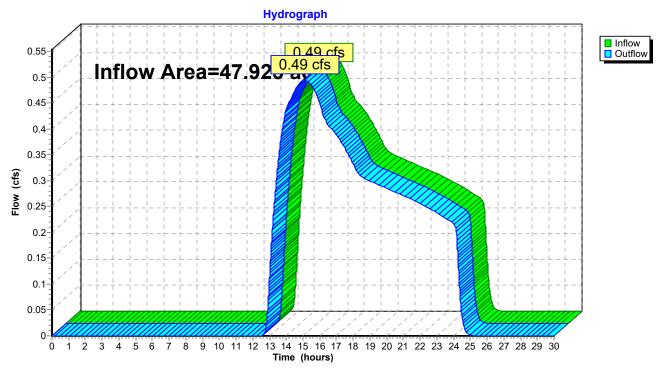
47.923 ac, 7.45% Impervious, Inflow Depth = 0.08" for 10-year Storm event Inflow Area =

Inflow 0.49 cfs @ 15.12 hrs, Volume= 0.310 af

Outflow 0.49 cfs @ 15.12 hrs, Volume= 0.310 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

## Reach DP: Design Point



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## Summary for Pond 1P: Recharge System A

3.638 ac, 9.45% Impervious, Inflow Depth = 0.14" for 10-year Storm event Inflow Area = 0.07 cfs @ 13.95 hrs, Volume= Inflow 0.043 af 0.07 cfs @ 14.00 hrs, Volume= Outflow = 0.043 af, Atten= 0%, Lag= 2.6 min 0.07 cfs @ 14.00 hrs, Volume= Discarded = 0.043 af Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach DP : Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 164.02' @ 14.00 hrs Surf.Area= 1,309 sf Storage= 10 cf

Plug-Flow detention time= 2.5 min calculated for 0.043 af (100% of inflow)

Center-of-Mass det. time= 2.5 min ( 1,051.3 - 1,048.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	164.50'	1,608 cf	ADS StormTech SC-740 b +Cap x 35 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	164.00'	1,189 cf	25.25'W x 51.84'L x 3.50'H Prismatoid
			4,581 cf Overall - 1,608 cf Embedded = 2,973 cf x 40.0% Voids
•		0.707. (	T

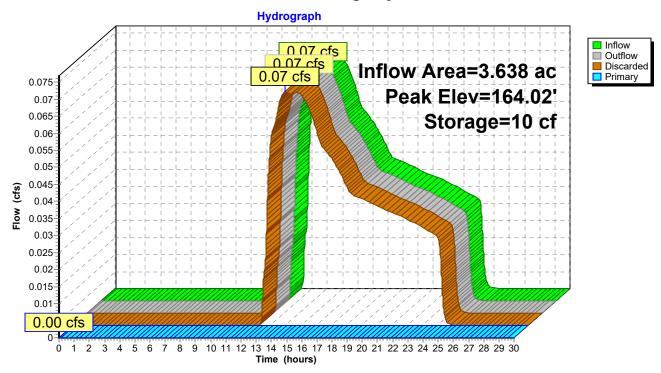
2,797 cf Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	164.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 160.00'
#2	Primary	171.25'	6.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

Discarded OutFlow Max=0.25 cfs @ 14.00 hrs HW=164.02' (Free Discharge) 1=Exfiltration (Controls 0.25 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=164.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

# Pond 1P: Recharge System A



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## Summary for Pond 2P: Recharge System B

Inflow Area = 7.186 ac, 9.69% Impervious, Inflow Depth = 0.21" for 10-year Storm event 0.32 cfs @ 12.47 hrs, Volume= Inflow 0.124 af 0.31 cfs @ 12.49 hrs, Volume= Outflow = 0.124 af, Atten= 1%, Lag= 1.6 min 0.31 cfs @ 12.49 hrs, Volume= Discarded = 0.124 af Primary = 0.000 af 0.00 cfs @ 0.00 hrs, Volume=

Routed to Reach DP : Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 162.02' @ 12.49 hrs Surf.Area= 3,257 sf Storage= 29 cf

Plug-Flow detention time= 1.5 min calculated for 0.124 af (100% of inflow)

Center-of-Mass det. time= 1.5 min ( 1,005.2 - 1,003.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	162.50'	4,135 cf	ADS_StormTech SC-740 b +Cap x 90 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	162.00'	2,907 cf	44.50'W x 73.20'L x 3.50'H Prismatoid
			11,401 cf Overall - 4,135 cf Embedded = 7,266 cf x 40.0% Voids
•			

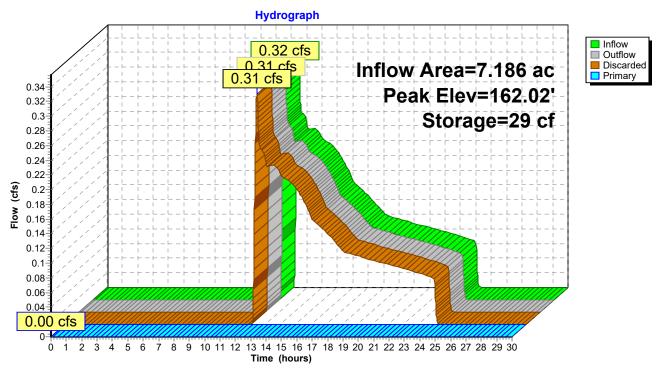
7,041 cf Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	162.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 160.00'
#2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

Discarded OutFlow Max=0.63 cfs @ 12.49 hrs HW=162.02' (Free Discharge) -1=Exfiltration (Controls 0.63 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=162.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

# Pond 2P: Recharge System B



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## Summary for Pond 3P: Recharge System C

Inflow Area = 1.060 ac, 39.30% Impervious, Inflow Depth = 1.32" for 10-year Storm event

1.45 cfs @ 12.11 hrs. Volume= Inflow 0.117 af

0.50 cfs @ 12.49 hrs, Volume= Outflow = 0.117 af, Atten= 65%, Lag= 22.6 min

0.50 cfs @ 12.49 hrs, Volume= Discarded = 0.117 af Primary 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach DP : Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 162.83' @ 12.49 hrs Surf.Area= 1,848 sf Storage= 877 cf

Plug-Flow detention time= 10.2 min calculated for 0.117 af (100% of inflow)

Center-of-Mass det. time= 10.2 min (883.5 - 873.3)

Volume	Invert	Avail.Storage	Storage Description
#1	162.50'	2,297 cf	ADS_StormTech SC-740 b +Cap x 50 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	162.00'	1,669 cf	25.25'W x 73.20'L x 3.50'H Prismatoid
-			6,469 cf Overall - 2,297 cf Embedded = 4,172 cf x 40.0% Voids

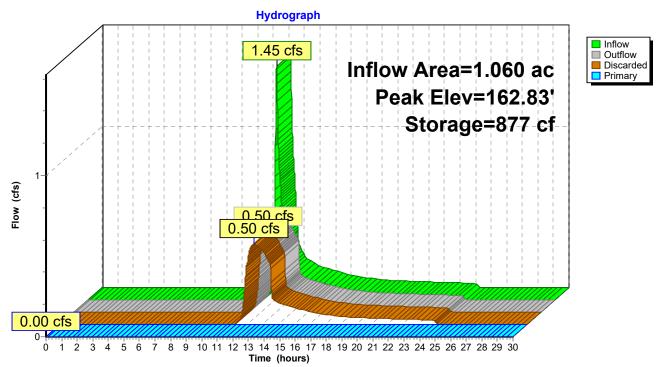
3,966 cf Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	162.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 160.00'
#2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

Discarded OutFlow Max=0.50 cfs @ 12.49 hrs HW=162.83' (Free Discharge) 1=Exfiltration (Controls 0.50 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=162.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

# Pond 3P: Recharge System C



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## Summary for Pond 4P: Recharge System D

Inflow Area = 1.273 ac, 41.36% Impervious, Inflow Depth = 1.39" for 10-year Storm event Inflow = 1.84 cfs @ 12.11 hrs, Volume= 0.147 af Outflow = 0.67 cfs @ 12.47 hrs, Volume= 0.147 af, Atten= 64%, Lag= 21.6 min Discarded = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach DP : Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 162.76' @ 12.47 hrs Surf.Area= 2,544 sf Storage= 1,050 cf

Plug-Flow detention time= 9.0 min calculated for 0.147 af (100% of inflow) Center-of-Mass det. time= 9.0 min (879.1 - 870.1)

Volume	Invert	Avail.Storage	Storage Description
#1	162.50'	3,216 cf	ADS_StormTech SC-740 b +Cap x 70 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	162.00'	2,275 cf	34.75'W x 73.20'L x 3.50'H Prismatoid
			8,903 cf Overall - 3,216 cf Embedded = 5,687 cf x 40.0% Voids

5,491 cf Total Available Storage

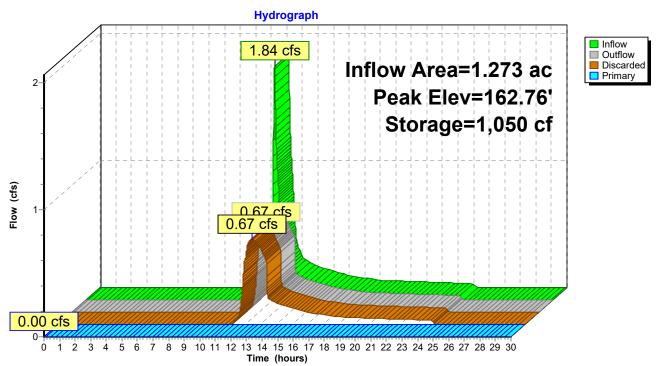
Devi	ce	Routing	Invert	Outlet Devices
#	1	Discarded	162.00'	8.270 in/hr Exfiltration over Surface area
				Conductivity to Groundwater Elevation = 160.00'
#	2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600
		·		Limited to weir flow at low heads

Discarded OutFlow Max=0.67 cfs @ 12.47 hrs HW=162.76' (Free Discharge)
1=Exfiltration (Controls 0.67 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=162.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

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# Pond 4P: Recharge System D



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# **Summary for Pond RAP1: Typical Roof Recharge Area 1**

Inflow Area =	0.030 ac,100.00% Impervious, Inflow Do	epth = 4.57" for 10-year Storm event
Inflow =	0.14 cfs @ 12.08 hrs, Volume=	0.011 af
Outflow =	0.04 cfs @ 12.39 hrs, Volume=	0.011 af, Atten= 69%, Lag= 18.4 min
Discarded =	0.04 cfs @ 12.39 hrs, Volume=	0.011 af
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 164.84' @ 12.39 hrs Surf.Area= 187 sf Storage= 82 cf

Plug-Flow detention time= 8.9 min calculated for 0.011 af (100% of inflow) Center-of-Mass det. time= 8.9 min (757.6 - 748.7)

Volume	Invert	Avail.Storage	Storage Description
#1	1 164.50' 88 cf		ADS_StormTech SC-310 x 6 Inside #2
			Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf
			Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap
#2	164.00'	139 cf	11.50'W x 16.24'L x 2.33'H Prismatoid
			435 cf Overall - 88 cf Embedded = 347 cf x 40.0% Voids
		227 cf	Total Available Storage

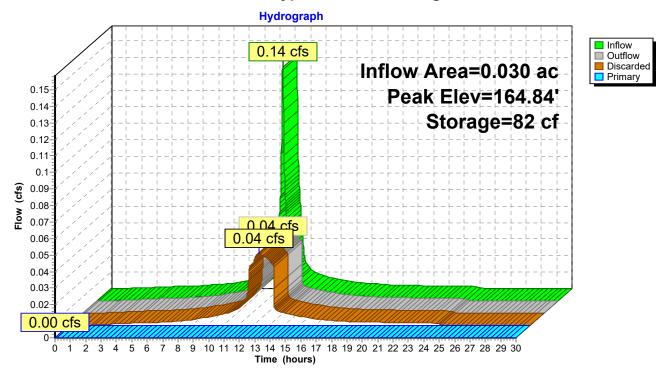
Device	Routing	Invert	Outlet Devices	
#1	Discarded	164.00'	8.270 in/hr Exfiltration over Surface area	
			Conductivity to Groundwater Elevation = 160.00'	
#2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600	
	•		Limited to weir flow at low heads	

**Discarded OutFlow** Max=0.04 cfs @ 12.39 hrs HW=164.84' (Free Discharge) **1=Exfiltration** (Controls 0.04 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=164.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

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# Pond RAP1: Typical Roof Recharge Area 1



# Pleasant Street Dunstable Post-Development-2Type III 24-hr 25-year Storm Rainfall=5.86" Prepared by Haley Ward Printed 9/11/2025

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

- Subcatchment 1S: Tributary Area to CB Runoff Area=123,030 sf 9.26% Impervious Runoff Depth=0.36" Flow Length=230' Slope=0.0500 '/' Tc=6.0 min CN=38 Runoff=0.34 cfs 0.084 af
- Subcatchment 2S: Tributary Area to CB 2A Runoff Area=35,455 sf 10.14% Impervious Runoff Depth=0.41" Flow Length=120' Slope=0.0200 '/' Tc=6.0 min CN=39 Runoff=0.12 cfs 0.028 af
- Subcatchment 3S: Tributary Area to CB 3A Runoff Area=97,380 sf 10.78% Impervious Runoff Depth=0.46" Flow Length=95' Slope=0.0200 '/' Tc=6.5 min CN=40 Runoff=0.42 cfs 0.085 af
- Subcatchment 4S: Tributary Area to CB 4A Runoff Area=166,140 sf 9.46% Impervious Runoff Depth=0.51" Flow Length=100' Slope=0.0200 '/' Tc=6.5 min CN=41 Runoff=0.87 cfs 0.163 af
- **Subcatchment 5S: Tributary Area to CB 5A** Runoff Area=49,501 sf 8.35% Impervious Runoff Depth=0.41" Flow Length=105' Slope=0.0200'/' Tc=6.6 min CN=39 Runoff=0.17 cfs 0.038 af
- Subcatchment 6S: Tributary Area to CB Runoff Area=46,170 sf 39.30% Impervious Runoff Depth=2.00" Flow Length=215' Slope=0.0200 '/' Tc=6.8 min CN=62 Runoff=2.30 cfs 0.176 af
- Subcatchment 7S: Tributary Area to CB 7A Runoff Area=55,449 sf 41.36% Impervious Runoff Depth=2.08" Flow Length=266' Slope=0.0200 '/' Tc=7.1 min CN=63 Runoff=2.87 cfs 0.221 af
- **Subcatchment 8S: Remaining Area**Runoff Area=1,514,422 sf 4.56% Impervious Runoff Depth=0.31"
  Flow Length=1,285' Tc=23.5 min CN=37 Runoff=2.32 cfs 0.896 af
- Subcatchment RA1: Typical Roof Area

  Runoff Area=1,310 sf 100.00% Impervious Runoff Depth=5.62"

  Tc=6.0 min CN=98 Runoff=0.17 cfs 0.014 af
- **Reach 1R: Prop. Drain DMH 1 to DMH 2** Avg. Flow Depth=0.25' Max Vel=2.23 fps Inflow=0.34 cfs 0.084 af 12.0" Round Pipe n=0.013 L=180.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=0.34 cfs 0.084 af
- **Reach 2R: Prop. Drain DMH 2 to DMH 3** Avg. Flow Depth=0.25' Max Vel=2.23 fps Inflow=0.34 cfs 0.084 af 12.0" Round Pipe n=0.013 L=180.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=0.34 cfs 0.084 af
- **Reach 3R: Prop. Drain DMH 3 to DMH 4** Avg. Flow Depth=0.29' Max Vel=2.42 fps Inflow=0.45 cfs 0.111 af 12.0" Round Pipe n=0.013 L=110.0' S=0.0050'/ Capacity=2.52 cfs Outflow=0.45 cfs 0.111 af
- **Reach 4R: Prop Drain DMH 4 to DMH 5** Avg. Flow Depth=0.29' Max Vel=2.42 fps Inflow=0.45 cfs 0.111 af 12.0" Round Pipe n=0.013 L=28.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=0.45 cfs 0.111 af
- **Reach 5R: Prop. Drain DMH x to DMH x** Avg. Flow Depth=0.59' Max Vel=4.82 fps Inflow=2.30 cfs 0.176 af 12.0" Round Pipe n=0.013 L=40.0' S=0.0100 '/' Capacity=3.56 cfs Outflow=2.30 cfs 0.176 af
- Reach DP: Design Point Inflow=2.32 cfs 0.896 af
  Outflow=2.32 cfs 0.896 af
- Pond 1P: Recharge System A Peak Elev=164.37' Storage=195 cf Inflow=0.45 cfs 0.111 af Discarded=0.27 cfs 0.111 af Primary=0.00 cfs 0.000 af Outflow=0.27 cfs 0.111 af

Pleasant Street Dunstable Post-Development-2Type III 24-hr 25-year Storm Rainfall=5.86"
Prepared by Haley Ward Printed 9/11/2025
HydroCAD® 10.20-6a s/n 00641 © 2024 HydroCAD Software Solutions LLC Page 62

Pond 2P: Recharge System B Peak Elev=162.58' Storage=879 cf Inflow=1.46 cfs 0.286 af Discarded=0.81 cfs 0.286 af Primary=0.00 cfs 0.000 af Outflow=0.81 cfs 0.286 af

Pond 3P: Recharge System C Peak Elev=163.46' Storage=1,797 cf Inflow=2.30 cfs 0.176 af Discarded=0.61 cfs 0.176 af Primary=0.00 cfs 0.000 af Outflow=0.61 cfs 0.176 af

Pond 4P: Recharge System D Peak Elev=163.30' Storage=2,166 cf Inflow=2.87 cfs 0.221 af Discarded=0.80 cfs 0.221 af Primary=0.00 cfs 0.000 af Outflow=0.80 cfs 0.221 af

Pond RAP1: Typical Roof Recharge Area 1 Peak Elev=165.12' Storage=119 cf Inflow=0.17 cfs 0.014 af Discarded=0.05 cfs 0.014 af Primary=0.00 cfs 0.000 af Outflow=0.05 cfs 0.014 af

Total Runoff Area = 47.954 ac Runoff Volume = 1.705 af Average Runoff Depth = 0.43" 92.50% Pervious = 44.355 ac 7.50% Impervious = 3.599 ac

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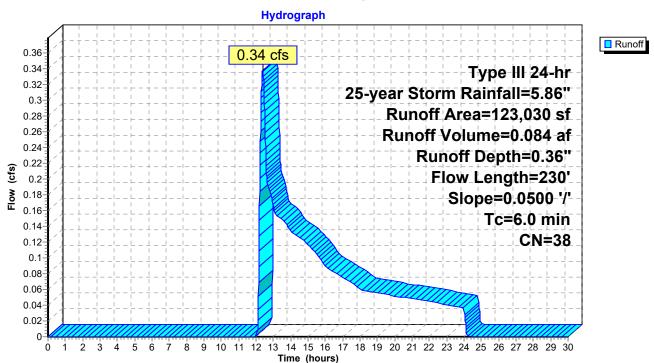
## Summary for Subcatchment 1S: Tributary Area to CB 1A-1B

Runoff = 0.34 cfs @ 12.38 hrs, Volume= 0.084 af, Depth= 0.36" Routed to Reach 1R : Prop. Drain DMH 1 to DMH 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-year Storm Rainfall=5.86"

_	A	rea (sf)	CN D	escription		
*		11,387			lriveways, s	
_	1	11,643	32 V	Voods/gras	ss comb., G	Good, HSG A
	1	23,030	38 V	Veighted A	verage	
	1	11,643	9	0.74% Per	vious Area	
		11,387	9	.26% Impe	rvious Area	a
	•					
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
	4.0	50	0.0500	0.21		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.12"
	0.7	180	0.0500	4.54		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	4.7	230	Total, I	ncreased t	o minimum	Tc = 6.0 min

#### Subcatchment 1S: Tributary Area to CB 1A-1B



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## Summary for Subcatchment 2S: Tributary Area to CB 2A & 2B

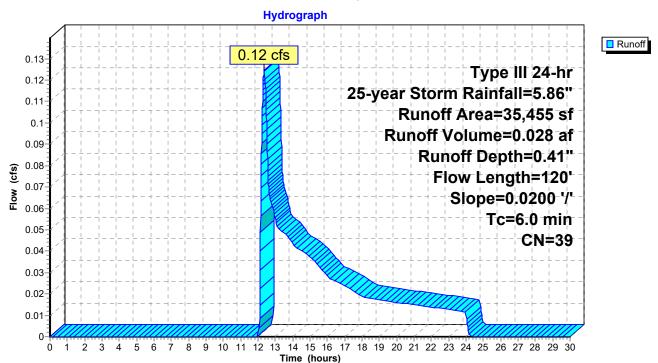
0.028 af, Depth= 0.41" Runoff 0.12 cfs @ 12.36 hrs, Volume= Routed to Reach 3R: Prop. Drain DMH 3 to DMH 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-year Storm Rainfall=5.86"

	Α	rea (sf)	CN E	escription		
		3,595	98 F	aved road	s w/curbs 8	k sewers, HSG A
		31,860	32 V	Voods/gras	s comb., G	Good, HSG A
		35,455	39 V	Veighted A	verage	
		31,860	8	9.86% Per	vious Area	
		3,595	1	0.14% Imp	ervious Are	ea
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	0.7	50	0.0200	1.18		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 3.12"
	0.4	70	0.0200	2.87		Shallow Concentrated Flow,
_						Paved Kv= 20.3 fps
	1.1	120	Total, I	ncreased t	o minimum	Tc = 6.0 min

Total, Increased to minimum Tc = 6.0 min

## Subcatchment 2S: Tributary Area to CB 2A & 2B



## Summary for Subcatchment 3S: Tributary Area to CB 3A & 3B

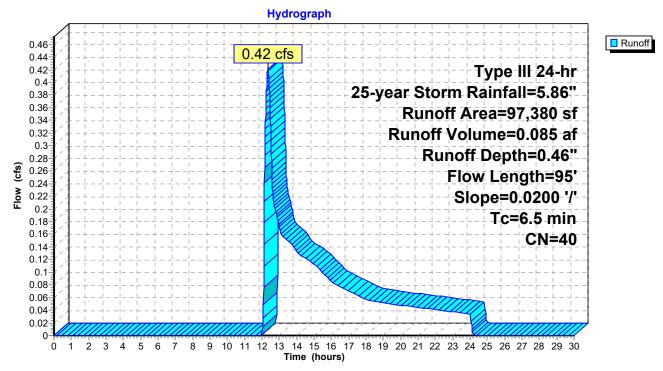
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Runoff = 0.42 cfs @ 12.34 hrs, Volume= 0.085 af, Depth= 0.46" Routed to Pond 2P : Recharge System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-year Storm Rainfall=5.86"

	Area (sf)	CN E	Description		
	10,499			ing, HSG A	
	15,431	39 >	·75% Gras	s cover, Go	ood, HSG A
	71,450	32 V	Voods/gras	ss comb., G	Good, HSG A
•	97,380	40 V	Veighted A	verage	
	86,881			vious Area	
	10,499	1	0.78% Imp	pervious Ar	ea
To	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
5.7	<sup>'</sup> 50	0.0200	0.15		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
0.8	3 45	0.0200	0.99		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
6.5	95	Total			<u> </u>

# Subcatchment 3S: Tributary Area to CB 3A & 3B



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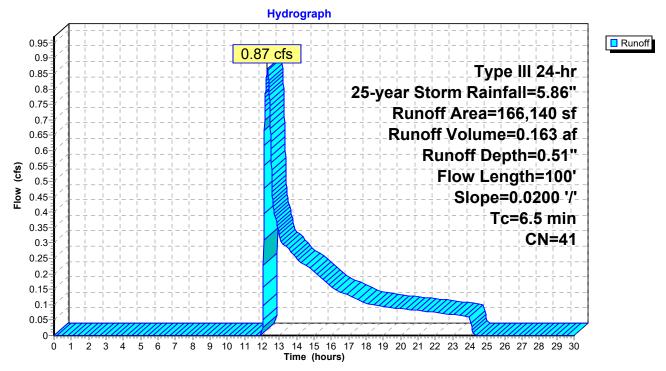
## Summary for Subcatchment 4S: Tributary Area to CB 4A & 4B

Runoff = 0.87 cfs @ 12.31 hrs, Volume= 0.163 af, Depth= 0.51" Routed to Pond 2P : Recharge System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-year Storm Rainfall=5.86"

A	rea (sf)	CN D	escription		
	15,714				R sewers, HSG A
	54,686	39 >	75% Gras	s cover, Go	ood, HSG A
	95,740	32 V	Voods/gras	ss comb., G	Good, HSG A
1	166,140	41 V	Veighted A	verage	
1	150,426	9	0.54% Per	vious Area	
	15,714	9	.46% Impe	ervious Area	a
			·		
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
5.7	50	0.0200	0.15		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
8.0	50	0.0200	0.99		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
6.5	100	Total			-

# Subcatchment 4S: Tributary Area to CB 4A & 4B



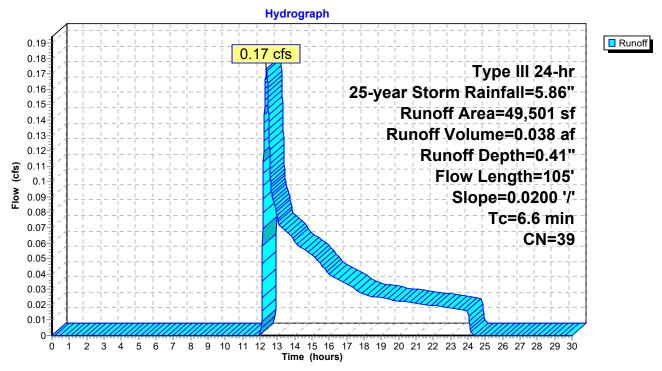
## Summary for Subcatchment 5S: Tributary Area to CB 5A & 5B

Runoff = 0.17 cfs @ 12.37 hrs, Volume= 0.038 af, Depth= 0.41" Routed to Pond 2P : Recharge System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-year Storm Rainfall=5.86"

	Α	rea (sf)	CN I	Description									
		4,134		98 Paved roads w/curbs & sewers, HSG A									
		9,382	39 :	>75% Gras	s cover, Go	ood, HSG A							
_		35,985	32 \	Woods/gras	ss comb., G	Good, HSG A							
		49,501	39 \	Weighted A	verage								
		45,367	,	91.65% Pei	rvious Area								
		4,134	;	3.35% Impe	ervious Are	a							
	_												
	Tc	Length	Slope		Capacity	Description							
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)								
	5.7	50	0.0200	0.15		Sheet Flow,							
						Grass: Short n= 0.150 P2= 3.12"							
	0.9	55	0.0200	0.99		Shallow Concentrated Flow,							
						Short Grass Pasture Kv= 7.0 fps							
	6.6	105	Total										

# Subcatchment 5S: Tributary Area to CB 5A & 5B



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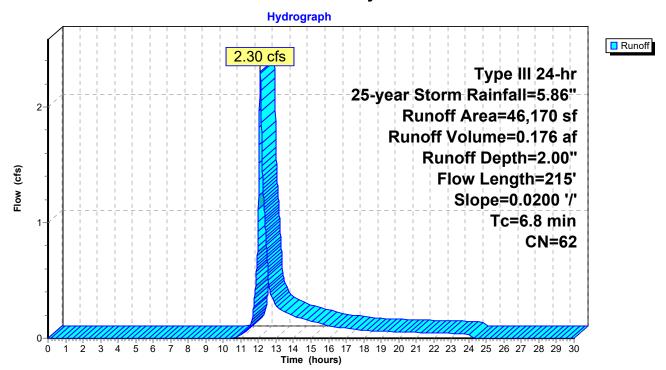
## Summary for Subcatchment 6S: Tributary Area to CB 6A-6B

Runoff = 2.30 cfs @ 12.11 hrs, Volume= 0.176 af, Depth= 2.00" Routed to Reach 5R : Prop. Drain DMH x to DMH x

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-year Storm Rainfall=5.86"

A	rea (sf)	CN E	Description			,				
	18,144		Paved roads w/curbs & sewers, HSG A							
	28,026	39 >	·75% Gras	s cover, Go	ood, HSG A					
	46,170	62 V	Veighted A	verage						
	28,026	6	0.70% Per	vious Area						
	18,144	3	9.30% Imp	ervious Ar	ea					
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
5.7	50	0.0200	0.15		Sheet Flow,					
					Grass: Short n= 0.150 P2= 3.12"					
0.6	80	0.0200	2.28		Shallow Concentrated Flow,					
					Unpaved Kv= 16.1 fps					
0.5	85	0.0200	2.87		Shallow Concentrated Flow,					
					Paved Kv= 20.3 fps					
6.8	215	Total								

## Subcatchment 6S: Tributary Area to CB 6A-6B



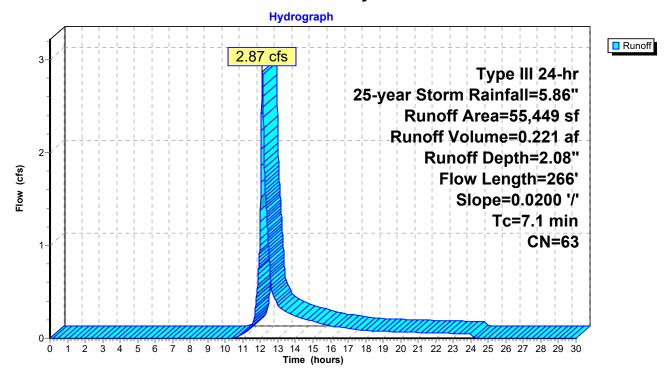
## Summary for Subcatchment 7S: Tributary Area to CB 7A & 7B

Runoff = 2.87 cfs @ 12.11 hrs, Volume= 0.221 af, Depth= 2.08" Routed to Pond 4P : Recharge System D

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-year Storm Rainfall=5.86"

/	Area (sf)	CN E	Description								
	22,933	98 F	Paved roads w/curbs & sewers, HSG A								
	32,516	39 >	75% Gras	s cover, Go	ood, HSG A						
	55,449	63 V	Veighted A	verage							
	32,516	5	8.64% Per	vious Area							
	22,933	4	1.36% Imp	ervious Ar	ea						
_											
Tc	Length	Slope	Velocity	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
5.7	50	0.0200	0.15		Sheet Flow,						
					Grass: Short n= 0.150 P2= 3.12"						
0.5	66	0.0200	2.28		Shallow Concentrated Flow,						
					Unpaved Kv= 16.1 fps						
0.9	150	0.0200	2.87		Shallow Concentrated Flow,						
					Paved Kv= 20.3 fps						
7.1	266	Total									

## Subcatchment 7S: Tributary Area to CB 7A & 7B



## Summary for Subcatchment 8S: Remaining Area Tributary to Design Point

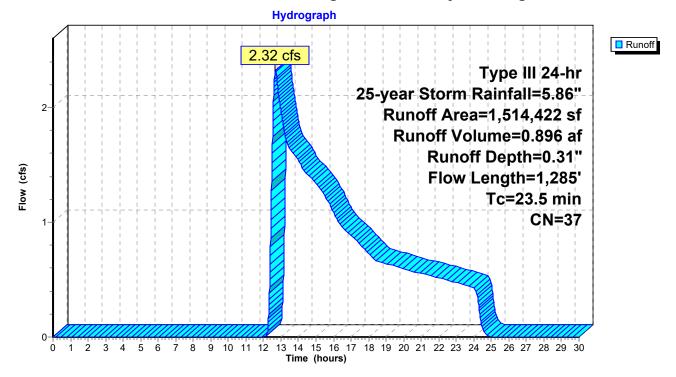
Runoff = 2.32 cfs @ 12.69 hrs, Volume= 0.896 af, Depth= 0.31"

Routed to Reach DP: Design Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-year Storm Rainfall=5.86"

A	rea (sf)	CN E	Description						
5	75,400	46 2	acre lots,	12% imp, F	HSG A				
1	11,580	49 5	0-75% Gra	ass cover, F	Fair, HSG A				
8	27,442	30 V	Voods, Go	od, HSG A					
1,5	14,422	37 V	Veighted A	verage					
1,4	45,374	9	5.44% Per	vious Area					
	69,048	4	.56% Impe	ervious Area	a				
Tc	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
15.1	50	0.0500	0.06		Sheet Flow,				
					Woods: Dense underbrush n= 0.800 P2= 3.12"				
5.4	725	0.2000	2.24		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
3.0	510	0.0300	2.79		Shallow Concentrated Flow,				
					Unpaved Kv= 16.1 fps				
23.5	1,285	Total							

## **Subcatchment 8S: Remaining Area Tributary to Design Point**



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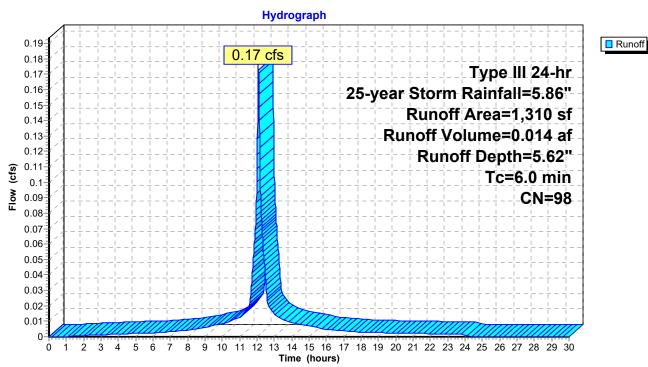
## **Summary for Subcatchment RA1: Typical Roof Area**

Runoff = 0.17 cfs @ 12.08 hrs, Volume= 0.014 af, Depth= 5.62" Routed to Pond RAP1 : Typical Roof Recharge Area 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-year Storm Rainfall=5.86"

_	Α	rea (sf)	CN [	Description						
		1,310	98 F	Roof Area						
		1,310	,	100.00% Impervious Area						
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
_	6.0					Direct Entry,				

## **Subcatchment RA1: Typical Roof Area**



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## Summary for Reach 1R: Prop. Drain DMH 1 to DMH 2

Inflow Area = 2.824 ac, 9.26% Impervious, Inflow Depth = 0.36" for 25-year Storm event

Inflow = 0.34 cfs @ 12.38 hrs, Volume= 0.084 af

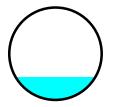
Outflow = 0.34 cfs @ 12.42 hrs, Volume= 0.084 af, Atten= 0%, Lag= 2.4 min

Routed to Reach 2R: Prop. Drain DMH 2 to DMH 3

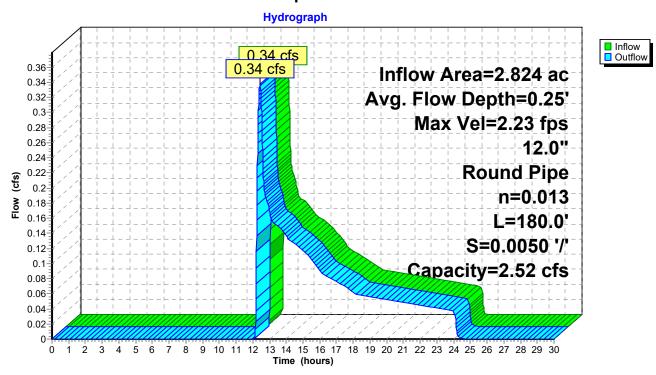
Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 2.23 fps, Min. Travel Time= 1.3 min Avg. Velocity = 1.37 fps, Avg. Travel Time= 2.2 min

Peak Storage= 27 cf @ 12.40 hrs Average Depth at Peak Storage= 0.25', Surface Width= 0.86' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe n= 0.013 Length= 180.0' Slope= 0.0050 '/' Inlet Invert= 168.00', Outlet Invert= 167.10'



#### Reach 1R: Prop. Drain DMH 1 to DMH 2



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## Summary for Reach 2R: Prop. Drain DMH 2 to DMH 3

Inflow Area = 2.824 ac, 9.26% Impervious, Inflow Depth = 0.36" for 25-year Storm event

Inflow = 0.34 cfs @ 12.42 hrs, Volume= 0.084 af

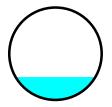
Outflow = 0.34 cfs @ 12.46 hrs, Volume= 0.084 af, Atten= 0%, Lag= 2.3 min

Routed to Reach 3R: Prop. Drain DMH 3 to DMH 4

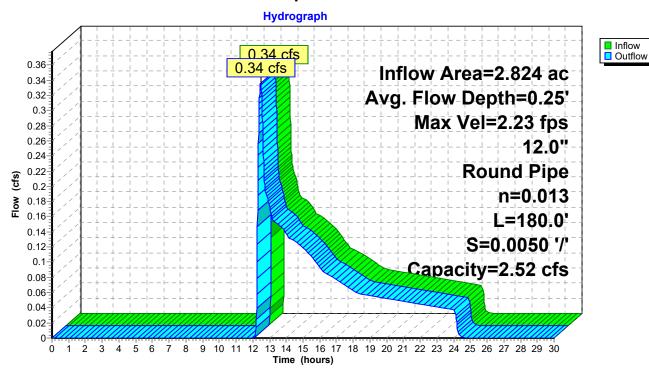
Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 2.23 fps, Min. Travel Time= 1.3 min Avg. Velocity = 1.35 fps, Avg. Travel Time= 2.2 min

Peak Storage= 27 cf @ 12.44 hrs Average Depth at Peak Storage= 0.25', Surface Width= 0.86' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe n= 0.013 Length= 180.0' Slope= 0.0050 '/' Inlet Invert= 167.00', Outlet Invert= 166.10'



#### Reach 2R: Prop. Drain DMH 2 to DMH 3



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## Summary for Reach 3R: Prop. Drain DMH 3 to DMH 4

Inflow Area = 3.638 ac, 9.45% Impervious, Inflow Depth = 0.37" for 25-year Storm event

Inflow = 0.45 cfs @ 12.44 hrs, Volume= 0.111 af

Outflow = 0.45 cfs @ 12.46 hrs, Volume= 0.111 af, Atten= 0%, Lag= 1.3 min

Routed to Reach 4R: Prop Drain DMH 4 to DMH 5

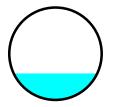
Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 2.42 fps, Min. Travel Time= 0.8 min

Avg. Velocity = 1.45 fps, Avg. Travel Time= 1.3 min

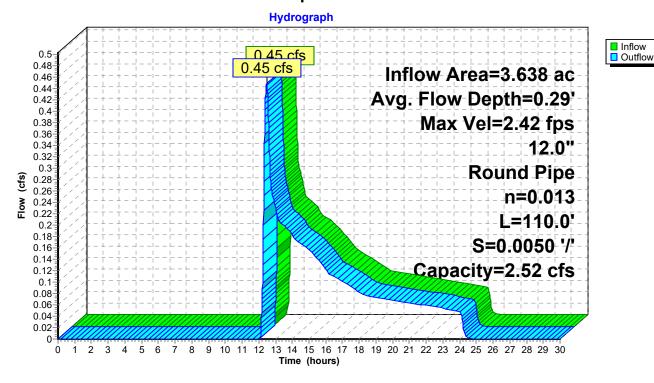
Peak Storage= 20 cf @ 12.45 hrs

Average Depth at Peak Storage= 0.29', Surface Width= 0.90' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe n= 0.013 Length= 110.0' Slope= 0.0050 '/' Inlet Invert= 166.00', Outlet Invert= 165.45'



#### Reach 3R: Prop. Drain DMH 3 to DMH 4



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#### Summary for Reach 4R: Prop Drain DMH 4 to DMH 5

Inflow Area = 3.638 ac, 9.45% Impervious, Inflow Depth = 0.37" for 25-year Storm event

Inflow = 0.45 cfs @ 12.46 hrs, Volume= 0.111 af

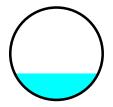
Outflow = 0.45 cfs @ 12.47 hrs, Volume= 0.111 af, Atten= 0%, Lag= 0.3 min

Routed to Pond 1P: Recharge System A

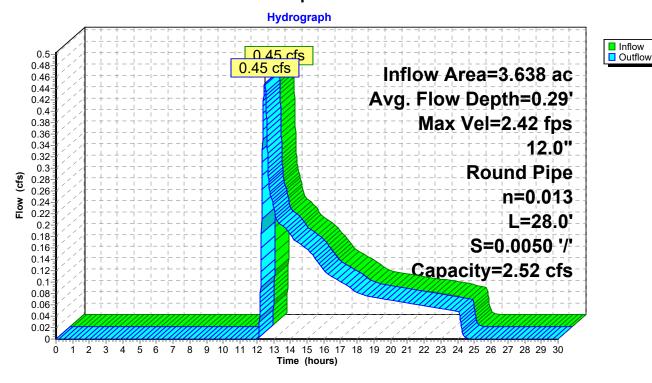
Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 2.42 fps, Min. Travel Time= 0.2 min Avg. Velocity = 1.45 fps, Avg. Travel Time= 0.3 min

Peak Storage= 5 cf @ 12.46 hrs Average Depth at Peak Storage= 0.29', Surface Width= 0.90' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 28.0' Slope= 0.0050 '/' Inlet Invert= 165.35', Outlet Invert= 165.21'



#### Reach 4R: Prop Drain DMH 4 to DMH 5



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## Summary for Reach 5R: Prop. Drain DMH x to DMH x

Inflow Area = 1.060 ac, 39.30% Impervious, Inflow Depth = 2.00" for 25-year Storm event

Inflow = 2.30 cfs @ 12.11 hrs, Volume= 0.176 af

Outflow = 2.30 cfs @ 12.11 hrs, Volume= 0.176 af, Atten= 0%, Lag= 0.3 min

Routed to Pond 3P: Recharge System C

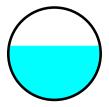
Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 4.82 fps, Min. Travel Time= 0.1 min

Avg. Velocity = 1.95 fps, Avg. Travel Time= 0.3 min

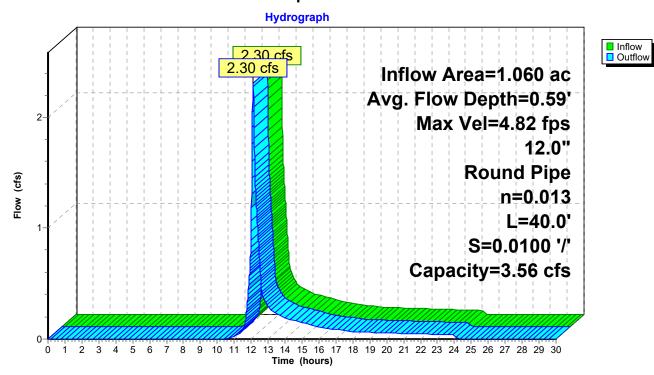
Peak Storage= 19 cf @ 12.11 hrs

Average Depth at Peak Storage= 0.59', Surface Width= 0.99' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 3.56 cfs

12.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 40.0' Slope= 0.0100 '/' Inlet Invert= 164.40', Outlet Invert= 164.00'



#### Reach 5R: Prop. Drain DMH x to DMH x



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# **Summary for Reach DP: Design Point**

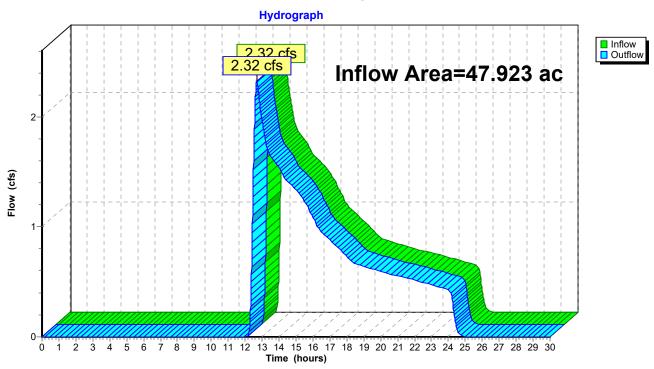
Inflow Area = 47.923 ac, 7.45% Impervious, Inflow Depth = 0.22" for 25-year Storm event

Inflow = 2.32 cfs @ 12.69 hrs, Volume= 0.896 af

Outflow = 2.32 cfs @ 12.69 hrs, Volume= 0.896 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

# Reach DP: Design Point



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## Summary for Pond 1P: Recharge System A

Inflow Area = 3.638 ac, 9.45% Impervious, Inflow Depth = 0.37" for 25-year Storm event Inflow = 0.45 cfs @ 12.47 hrs, Volume= 0.111 af Outflow = 0.27 cfs @ 12.71 hrs, Volume= 0.111 af, Atten= 39%, Lag= 14.8 min Discarded = 0.027 cfs @ 12.71 hrs, Volume= 0.111 af Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach DP : Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 164.37' @ 12.71 hrs Surf.Area= 1,309 sf Storage= 195 cf

Plug-Flow detention time= 4.0 min calculated for 0.111 af (100% of inflow) Center-of-Mass det. time= 4.0 min (983.9 - 980.0)

Volume	Invert	Avail.Storage	Storage Description
#1	#1 164.50' 1,608 cf		ADS_StormTech SC-740 b +Cap x 35 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2 164.00' 1,189		1,189 cf	25.25'W x 51.84'L x 3.50'H Prismatoid
			4,581 cf Overall - 1,608 cf Embedded = 2,973 cf x 40.0% Voids
		0.707 . (	Total Accellable Otomore

2,797 cf Total Available Storage

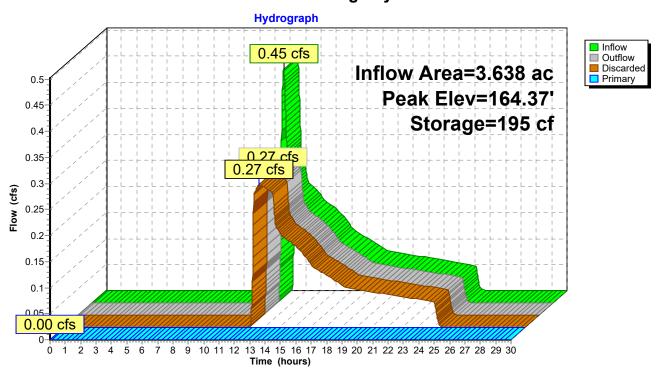
Device	Routing	Invert	Outlet Devices	
#1	Discarded	164.00'	8.270 in/hr Exfiltration over Surface area	
			Conductivity to Groundwater Elevation = 160.00'	
#2	Primary	171.25'	6.0" Horiz. Orifice/Grate C= 0.600	
	•		Limited to weir flow at low heads	

Discarded OutFlow Max=0.27 cfs @ 12.71 hrs HW=164.37' (Free Discharge) 1=Exfiltration (Controls 0.27 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=164.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

Pond 1P: Recharge System A

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#### Pleasant Street Dunstable Post-Development-2Type III 24-hr 25-year Storm Rainfall=5.86" Printed 9/11/2025

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## Summary for Pond 2P: Recharge System B

Inflow Area = 7.186 ac, 9.69% Impervious, Inflow Depth = 0.48" for 25-year Storm event 1.46 cfs @ 12.33 hrs, Volume= Inflow 0.286 af 0.81 cfs @ 12.59 hrs, Volume= Outflow = 0.286 af, Atten= 45%, Lag= 15.6 min 0.81 cfs @ 12.59 hrs, Volume= Discarded = 0.286 af Primary 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach DP: Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 162.58' @ 12.59 hrs Surf.Area= 3,257 sf Storage= 879 cf

Plug-Flow detention time= 5.2 min calculated for 0.286 af (100% of inflow) Center-of-Mass det. time= 5.2 min ( 957.8 - 952.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	162.50'	4,135 cf	ADS_StormTech SC-740 b +Cap x 90 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	162.00'	2,907 cf	44.50'W x 73.20'L x 3.50'H Prismatoid
			11,401 cf Overall - 4,135 cf Embedded = 7,266 cf x 40.0% Voids

7,041 cf Total Available Storage

Device	Routing	Invert	Outlet Devices	
#1	Discarded	162.00'	8.270 in/hr Exfiltration over Surface area	
			Conductivity to Groundwater Elevation = 160.00'	
#2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600	
	-		I imited to weir flow at low heads	

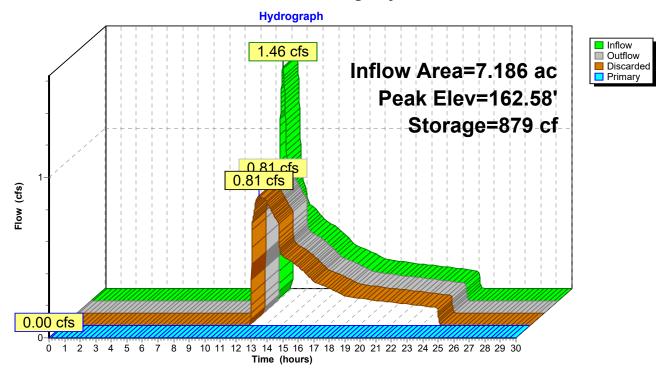
Discarded OutFlow Max=0.81 cfs @ 12.59 hrs HW=162.58' (Free Discharge) 1=Exfiltration (Controls 0.81 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=162.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

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# Pond 2P: Recharge System B



# Pleasant Street Dunstable Post-Development-2Type III 24-hr 25-year Storm Rainfall=5.86"

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## Summary for Pond 3P: Recharge System C

Discarded = 0.61 cfs @ 12.54 hrs, Volume= 0.176 af Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach DP : Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 163.46' @ 12.54 hrs Surf.Area= 1,848 sf Storage= 1,797 cf

Plug-Flow detention time= 20.2 min calculated for 0.176 af (100% of inflow)

Center-of-Mass det. time= 20.2 min ( 880.4 - 860.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	162.50'	2,297 cf	ADS_StormTech SC-740 b +Cap x 50 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	162.00'	1,669 cf	25.25'W x 73.20'L x 3.50'H Prismatoid
			6,469 cf Overall - 2,297 cf Embedded = 4,172 cf x 40.0% Voids
		2 2 2 2 5	<b>-</b>

3,966 cf Total Available Storage

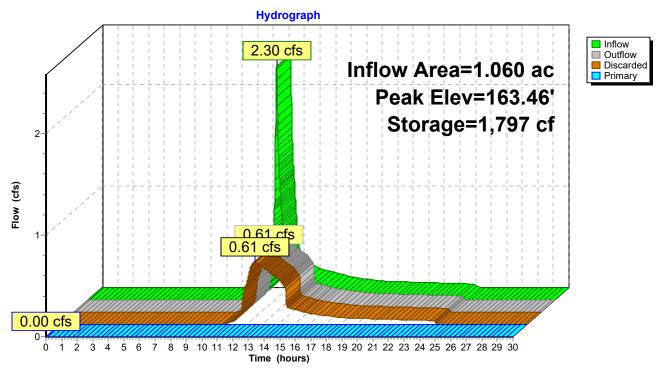
Device	Routing	Invert	Outlet Devices	
#1	Discarded	162.00'	8.270 in/hr Exfiltration over Surface area	
			Conductivity to Groundwater Elevation = 160.00'	
#2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600	
	•		Limited to weir flow at low heads	

Discarded OutFlow Max=0.61 cfs @ 12.54 hrs HW=163.46' (Free Discharge)
1=Exfiltration (Controls 0.61 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=162.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

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# Pond 3P: Recharge System C



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## Summary for Pond 4P: Recharge System D

Inflow Area = 1.273 ac, 41.36% Impervious, Inflow Depth = 2.08" for 25-year Storm event Inflow 2.87 cfs @ 12.11 hrs. Volume= 0.221 af 0.80 cfs @ 12.52 hrs, Volume= Outflow = 0.221 af, Atten= 72%, Lag= 24.8 min 0.80 cfs @ 12.52 hrs, Volume= Discarded = 0.221 af Primary 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach DP : Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 163.30' @ 12.52 hrs Surf.Area= 2,544 sf Storage= 2,166 cf

Plug-Flow detention time= 17.8 min calculated for 0.220 af (100% of inflow) Center-of-Mass det. time= 17.8 min (875.4 - 857.5)

Volume	Invert	Avail.Storage	Storage Description
#1	162.50'	3,216 cf	ADS_StormTech SC-740 b +Cap x 70 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	162.00'	2,275 cf	34.75'W x 73.20'L x 3.50'H Prismatoid
			8,903 cf Overall - 3,216 cf Embedded = 5,687 cf x 40.0% Voids

5,491 cf Total Available Storage

Devi	ce	Routing	Invert	Outlet Devices
#	1	Discarded	162.00'	8.270 in/hr Exfiltration over Surface area
				Conductivity to Groundwater Elevation = 160.00'
#	2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600
		·		Limited to weir flow at low heads

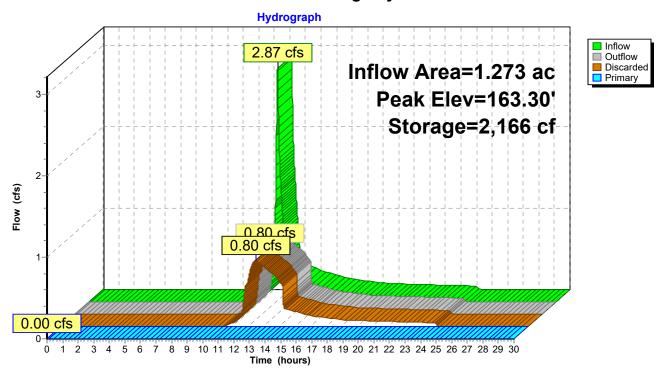
Discarded OutFlow Max=0.80 cfs @ 12.52 hrs HW=163.30' (Free Discharge) -1=Exfiltration (Controls 0.80 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=162.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

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# Pond 4P: Recharge System D



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## Summary for Pond RAP1: Typical Roof Recharge Area 1

Inflow Area = 0.030 ac,100.00% Impervious, Inflow Depth = 5.62" for 25-year Storm event 
Inflow = 0.17 cfs @ 12.08 hrs, Volume= 0.014 af 
Outflow = 0.05 cfs @ 12.43 hrs, Volume= 0.014 af, Atten= 73%, Lag= 21.1 min 
Discarded = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 165.12' @ 12.43 hrs Surf.Area= 187 sf Storage= 119 cf

Plug-Flow detention time= 12.8 min calculated for 0.014 af (100% of inflow) Center-of-Mass det. time= 12.8 min (758.3 - 745.5)

Volume	Invert	Avail.Storage	Storage Description
#1 164.50'		88 cf	ADS_StormTech SC-310 x 6 Inside #2
			Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf
			Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap
#2	164.00'	139 cf	11.50'W x 16.24'L x 2.33'H Prismatoid
			435 cf Overall - 88 cf Embedded = 347 cf x 40.0% Voids
		227 cf	Total Available Storage

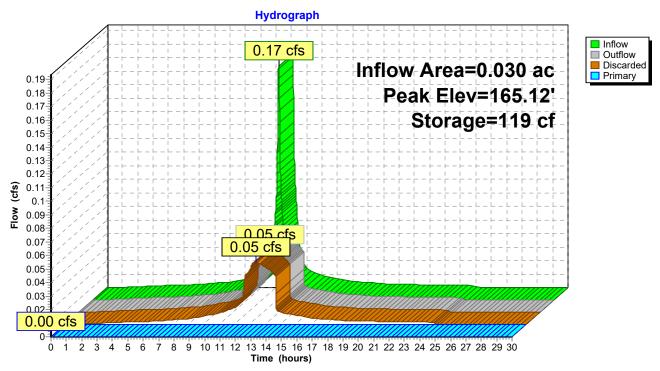
Device	Routing	Invert	Outlet Devices	
#1	Discarded	164.00'	8.270 in/hr Exfiltration over Surface area	
			Conductivity to Groundwater Elevation = 160.00'	
#2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600	
	•		Limited to weir flow at low heads	

Discarded OutFlow Max=0.05 cfs @ 12.43 hrs HW=165.12' (Free Discharge) 1=Exfiltration (Controls 0.05 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=164.00' (Free Discharge) 2=Orifice/Grate (Controls 0.00 cfs)

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# Pond RAP1: Typical Roof Recharge Area 1



# Pleasant Street Dunstable Post-Development-Type III 24-hr 100-year Storm Rainfall=7.48" Prepared by Haley Ward Printed 9/11/2025

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

- Subcatchment 1S: Tributary Area to CB Runoff Area=123,030 sf 9.26% Impervious Runoff Depth=0.87" Flow Length=230' Slope=0.0500 '/' Tc=6.0 min CN=38 Runoff=1.47 cfs 0.204 af
- Subcatchment 2S: Tributary Area to CB 2A Runoff Area=35,455 sf 10.14% Impervious Runoff Depth=0.95" Flow Length=120' Slope=0.0200 '/' Tc=6.0 min CN=39 Runoff=0.51 cfs 0.064 af
- Subcatchment 3S: Tributary Area to CB 3A Runoff Area=97,380 sf 10.78% Impervious Runoff Depth=1.03" Flow Length=95' Slope=0.0200 '/' Tc=6.5 min CN=40 Runoff=1.64 cfs 0.192 af
- Subcatchment 4S: Tributary Area to CB 4A Runoff Area=166,140 sf 9.46% Impervious Runoff Depth=1.12" Flow Length=100' Slope=0.0200 '/' Tc=6.5 min CN=41 Runoff=3.26 cfs 0.354 af
- Subcatchment 5S: Tributary Area to CB 5A Runoff Area=49,501 sf 8.35% Impervious Runoff Depth=0.95" Flow Length=105' Slope=0.0200'/' Tc=6.6 min CN=39 Runoff=0.70 cfs 0.090 af
- **Subcatchment 6S: Tributary Area to CB**Flow Length=215' Runoff Area=46,170 sf 39.30% Impervious Runoff Depth=3.16"
  Slope=0.0200 '/' Tc=6.8 min CN=62 Runoff=3.76 cfs 0.279 af
- Subcatchment 7S: Tributary Area to CB 7A Runoff Area=55,449 sf 41.36% Impervious Runoff Depth=3.26" Flow Length=266' Slope=0.0200 '/' Tc=7.1 min CN=63 Runoff=4.64 cfs 0.346 af
- Subcatchment 8S: Remaining Area

  Runoff Area=1,514,422 sf 4.56% Impervious Runoff Depth=0.79"

  Flow Length=1,285' Tc=23.5 min CN=37 Runoff=11.28 cfs 2.279 af
- Subcatchment RA1: Typical Roof Area

  Runoff Area=1,310 sf 100.00% Impervious Runoff Depth=7.24"

  Tc=6.0 min CN=98 Runoff=0.22 cfs 0.018 af
- **Reach 1R: Prop. Drain DMH 1 to DMH 2** Avg. Flow Depth=0.54' Max Vel=3.32 fps Inflow=1.47 cfs 0.204 af 12.0" Round Pipe n=0.013 L=180.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=1.45 cfs 0.204 af
- **Reach 2R: Prop. Drain DMH 2 to DMH 3** Avg. Flow Depth=0.54' Max Vel=3.31 fps Inflow=1.45 cfs 0.204 af 12.0" Round Pipe n=0.013 L=180.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=1.44 cfs 0.204 af
- **Reach 3R: Prop. Drain DMH 3 to DMH 4** Avg. Flow Depth=0.64' Max Vel=3.52 fps Inflow=1.89 cfs 0.268 af 12.0" Round Pipe n=0.013 L=110.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=1.88 cfs 0.268 af
- **Reach 4R: Prop Drain DMH 4 to DMH 5** Avg. Flow Depth=0.64' Max Vel=3.52 fps Inflow=1.88 cfs 0.268 af 12.0" Round Pipe n=0.013 L=28.0' S=0.0050 '/' Capacity=2.52 cfs Outflow=1.88 cfs 0.268 af
- **Reach 5R: Prop. Drain DMH x to DMH x** Avg. Flow Depth=0.88' Max Vel=5.17 fps Inflow=3.76 cfs 0.279 af 12.0" Round Pipe n=0.013 L=40.0' S=0.0100 '/' Capacity=3.56 cfs Outflow=3.76 cfs 0.279 af
- Reach DP: Design Point Inflow=11.28 cfs 2.279 af
  Outflow=11.28 cfs 2.279 af
- Pond 1P: Recharge System A Peak Elev=167.04' Storage=2,557 cf Inflow=1.88 cfs 0.268 af Discarded=0.44 cfs 0.268 af Primary=0.00 cfs 0.000 af Outflow=0.44 cfs 0.268 af

Pleasant Street Dunstable Post-Development-Type III 24-hr 100-year Storm Rainfall=7.48"
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Pond 2P: Recharge System B Peak Elev=164.57' Storage=5,718 cf Inflow=5.59 cfs 0.636 af Discarded=1.42 cfs 0.636 af Primary=0.00 cfs 0.000 af Outflow=1.42 cfs 0.636 af

Pond 3P: Recharge System C Peak Elev=164.84' Storage=3,470 cf Inflow=3.76 cfs 0.279 af Discarded=0.86 cfs 0.279 af Primary=0.00 cfs 0.000 af Outflow=0.86 cfs 0.279 af

Pond 4P: Recharge System D Peak Elev=164.41' Storage=4,214 cf Inflow=4.64 cfs 0.346 af Discarded=1.07 cfs 0.346 af Primary=0.00 cfs 0.000 af Outflow=1.07 cfs 0.346 af

Pond RAP1: Typical Roof Recharge Area 1 Peak Elev=165.67' Storage=177 cf Inflow=0.22 cfs 0.018 af Discarded=0.05 cfs 0.018 af Primary=0.00 cfs 0.000 af Outflow=0.05 cfs 0.018 af

Total Runoff Area = 47.954 ac Runoff Volume = 3.827 af Average Runoff Depth = 0.96" 92.50% Pervious = 44.355 ac 7.50% Impervious = 3.599 ac

## Summary for Subcatchment 1S: Tributary Area to CB 1A-1B

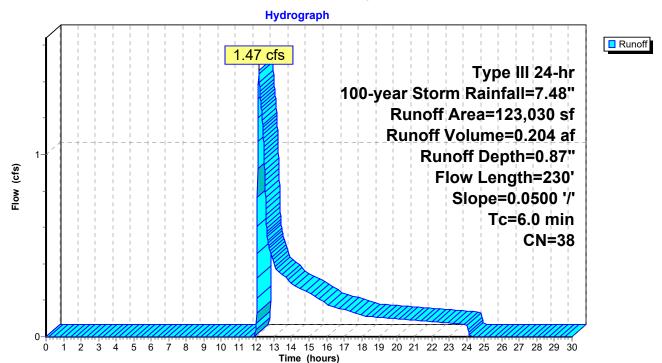
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Runoff = 1.47 cfs @ 12.14 hrs, Volume= 0.204 af, Depth= 0.87" Routed to Reach 1R : Prop. Drain DMH 1 to DMH 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Storm Rainfall=7.48"

	Α	rea (sf)	CN D	escription		
*		11,387	98 R	loadway, d	lriveways, s	sidewalks
_	1	11,643	32 V	Voods/gras	ss comb., G	Good, HSG A
123,030 38 Weighted Average			Veighted A	verage		
	1	11,643	9	0.74% Per	vious Area	
11,387 9.26% Impervious Area					a	
	_				_	
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.0	50	0.0500	0.21		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.12"
	0.7	180	0.0500	4.54		Shallow Concentrated Flow,
_						Paved Kv= 20.3 fps
	4.7	230	Total, I	ncreased t	o minimum	Tc = 6.0 min

## **Subcatchment 1S: Tributary Area to CB 1A-1B**



#### Summary for Subcatchment 2S: Tributary Area to CB 2A & 2B

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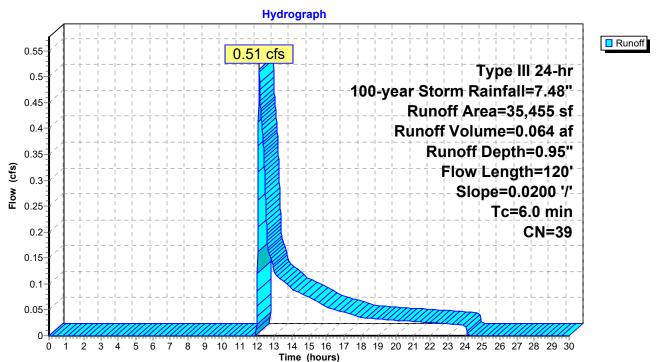
Runoff = 0.51 cfs @ 12.13 hrs, Volume= 0.064 af, Depth= 0.95" Routed to Reach 3R : Prop. Drain DMH 3 to DMH 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Storm Rainfall=7.48"

_	Α	rea (sf)	CN E	CN Description				
		3,595	98 F	aved road	s w/curbs 8	& sewers, HSG A		
_		31,860	32 V	Voods/gras	s comb., G	Good, HSG A		
		35,455	39 V	Veighted A	verage			
		31,860	8	9.86% Per	vious Area			
	3,595 10.14% Impervious Area					ea		
	_							
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	0.7	50	0.0200	1.18		Sheet Flow,		
						Smooth surfaces n= 0.011 P2= 3.12"		
	0.4	70	0.0200	2.87		Shallow Concentrated Flow,		
_						Paved Kv= 20.3 fps		
	1 1	120	Total I	naragaed t	a minimum	To = 6.0 min		

1.1 120 Total, Increased to minimum Tc = 6.0 min

# Subcatchment 2S: Tributary Area to CB 2A & 2B



#### Summary for Subcatchment 3S: Tributary Area to CB 3A & 3B

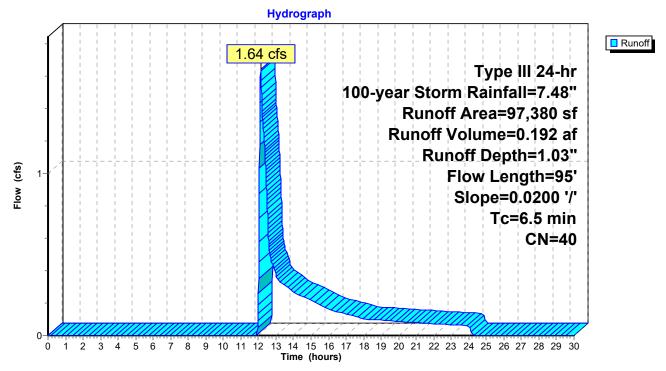
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Runoff = 1.64 cfs @ 12.13 hrs, Volume= 0.192 af, Depth= 1.03" Routed to Pond 2P : Recharge System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Storm Rainfall=7.48"

	rea (sf)	CN E	escription					
	10,499			ing, HSG A				
	15,431	39 >	75% Gras	s cover, Go	ood, HSG A			
	71,450	32 V	Voods/gras	ss comb., G	Good, HSG A			
	97,380	40 V	40 Weighted Average					
	86,881			vious Area				
	10,499	1	0.78% Imp	ervious Ar	ea			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·			
5.7	50	0.0200	0.15		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.12"			
0.8	45	0.0200	0.99		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
6.5	95	Total			<u> </u>			

## Subcatchment 3S: Tributary Area to CB 3A & 3B



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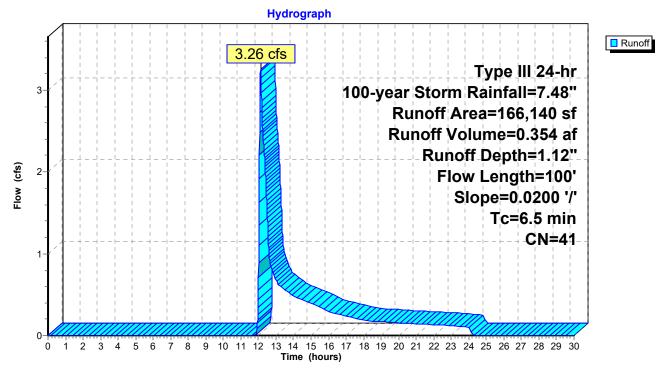
## Summary for Subcatchment 4S: Tributary Area to CB 4A & 4B

Runoff = 3.26 cfs @ 12.13 hrs, Volume= 0.354 af, Depth= 1.12" Routed to Pond 2P : Recharge System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Storm Rainfall=7.48"

	Area (sf)	CN D	escription		
	15,714				& sewers, HSG A
	54,686	39 >	75% Gras	s cover, Go	ood, HSG A
	95,740	32 V	Voods/gras	ss comb., G	Good, HSG A
	166,140	41 V	Veighted A	verage	
	150,426	9	0.54% Per	vious Area	
	15,714	9.46% Impervious Area			a
			-		
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
5.7	50	0.0200	0.15		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
0.8	50	0.0200	0.99		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
6.5	100	Total			<u> </u>

## Subcatchment 4S: Tributary Area to CB 4A & 4B



#### Summary for Subcatchment 5S: Tributary Area to CB 5A & 5B

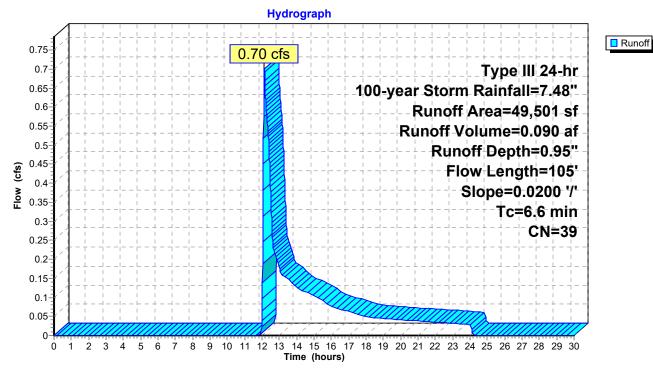
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Runoff = 0.70 cfs @ 12.14 hrs, Volume= 0.090 af, Depth= 0.95" Routed to Pond 2P : Recharge System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Storm Rainfall=7.48"

A	rea (sf)	CN D	escription					
	4,134				& sewers, HSG A			
	9,382	39 >	75% Gras	s cover, Go	ood, HSG A			
	35,985	32 V	Voods/gras	s comb., G	Good, HSG A			
	49,501	39 V	39 Weighted Average					
	45,367	9						
	4,134	8	.35% Impe	ervious Area	a			
			-					
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.7	50	0.0200	0.15		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.12"			
0.9	55	0.0200	0.99		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
6.6	105	Total						

# Subcatchment 5S: Tributary Area to CB 5A & 5B



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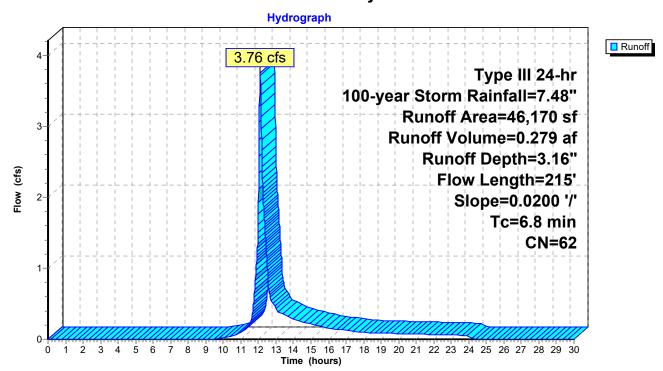
## Summary for Subcatchment 6S: Tributary Area to CB 6A-6B

Runoff = 3.76 cfs @ 12.10 hrs, Volume= 0.279 af, Depth= 3.16" Routed to Reach 5R : Prop. Drain DMH x to DMH x

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Storm Rainfall=7.48"

A	rea (sf)	CN E	Description			,
	18,144				& sewers, HSG A	
	28,026	39 >	·75% Gras	s cover, Go	ood, HSG A	
	46,170	62 V	Veighted A	verage		
	28,026	6	0.70% Per	vious Area		
	18,144	3	9.30% Imp	ervious Ar	ea	
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
5.7	50	0.0200	0.15		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.12"	
0.6	80	0.0200	2.28		Shallow Concentrated Flow,	
					Unpaved Kv= 16.1 fps	
0.5	85	0.0200	2.87		Shallow Concentrated Flow,	
					Paved Kv= 20.3 fps	
6.8	215	Total				

#### Subcatchment 6S: Tributary Area to CB 6A-6B



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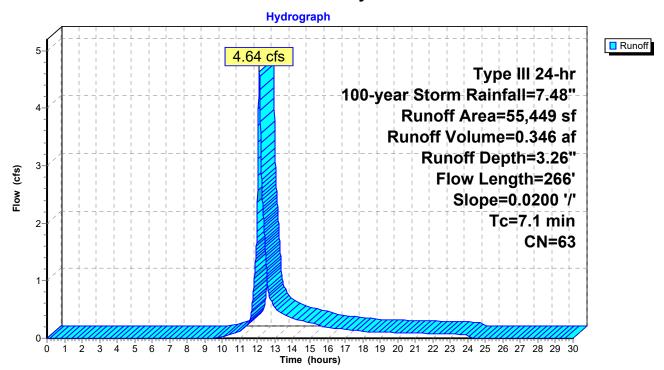
## Summary for Subcatchment 7S: Tributary Area to CB 7A & 7B

Runoff = 4.64 cfs @ 12.11 hrs, Volume= 0.346 af, Depth= 3.26" Routed to Pond 4P : Recharge System D

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Storm Rainfall=7.48"

A	rea (sf)	CN E	escription			
	22,933				& sewers, HSG A	
	32,516	39 >	75% Gras	s cover, Go	ood, HSG A	
	55,449	63 V	Veighted A	verage		
	32,516	5	8.64% Per	vious Area		
	22,933	4	1.36% Imp	ervious Ar	ea	
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
5.7	50	0.0200	0.15		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.12"	
0.5	66	0.0200	2.28		Shallow Concentrated Flow,	
					Unpaved Kv= 16.1 fps	
0.9	150	0.0200	2.87		Shallow Concentrated Flow,	
					Paved Kv= 20.3 fps	
7.1	266	Total				

# Subcatchment 7S: Tributary Area to CB 7A & 7B



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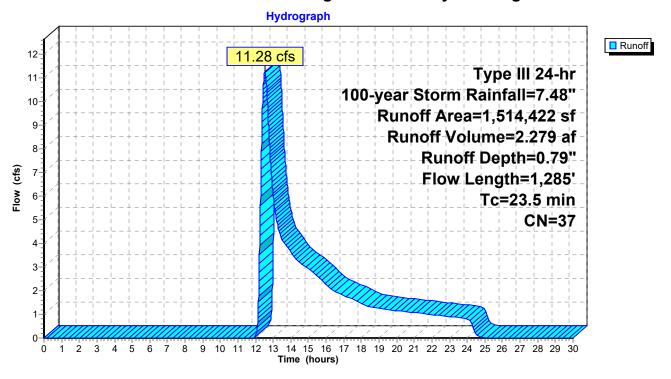
#### **Summary for Subcatchment 8S: Remaining Area Tributary to Design Point**

Runoff = 11.28 cfs @ 12.53 hrs, Volume= 2.279 af, Depth= 0.79" Routed to Reach DP : Design Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Storm Rainfall=7.48"

_	Α	rea (sf)	CN E	escription		
	5	75,400	46 2	acre lots,	12% imp, F	HSG A
	1	11,580	49 5	0-75% Gra	ass cover, F	Fair, HSG A
_	8	27,442	30 V	Voods, Go	od, HSG A	
	1,5	14,422	37 Weighted Average			
	1,4	45,374	9	5.44% Per	vious Area	
		69,048	4	.56% Impe	ervious Area	a
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.1	50	0.0500	0.06		Sheet Flow,
						Woods: Dense underbrush n= 0.800 P2= 3.12"
	5.4	725	0.2000	2.24		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	3.0	510	0.0300	2.79		Shallow Concentrated Flow,
_						Unpaved Kv= 16.1 fps
	23.5	1,285	Total			

#### **Subcatchment 8S: Remaining Area Tributary to Design Point**



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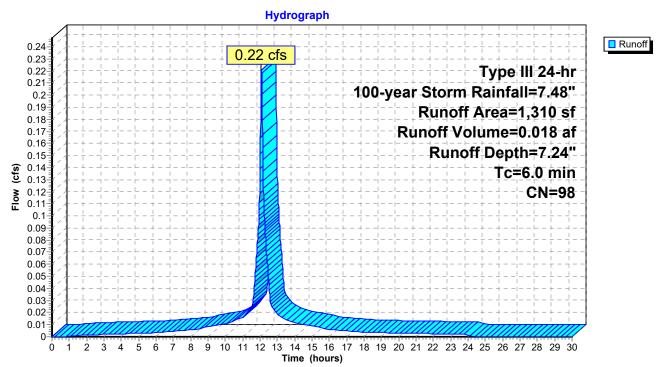
## **Summary for Subcatchment RA1: Typical Roof Area**

Runoff = 0.22 cfs @ 12.08 hrs, Volume= 0.018 af, Depth= 7.24" Routed to Pond RAP1 : Typical Roof Recharge Area 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Storm Rainfall=7.48"

	Α	rea (sf)	CN I	Description		
		1,310	98	Roof Area		
_		1,310	•	100.00% Im	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
_	6.0					Direct Entry.

## **Subcatchment RA1: Typical Roof Area**



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#### Summary for Reach 1R: Prop. Drain DMH 1 to DMH 2

Inflow Area = 2.824 ac, 9.26% Impervious, Inflow Depth = 0.87" for 100-year Storm event

Inflow = 1.47 cfs @ 12.14 hrs, Volume= 0.204 af

Outflow = 1.45 cfs @ 12.16 hrs, Volume= 0.204 af, Atten= 1%, Lag= 1.7 min

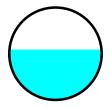
Routed to Reach 2R: Prop. Drain DMH 2 to DMH 3

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 3.32 fps, Min. Travel Time= 0.9 min

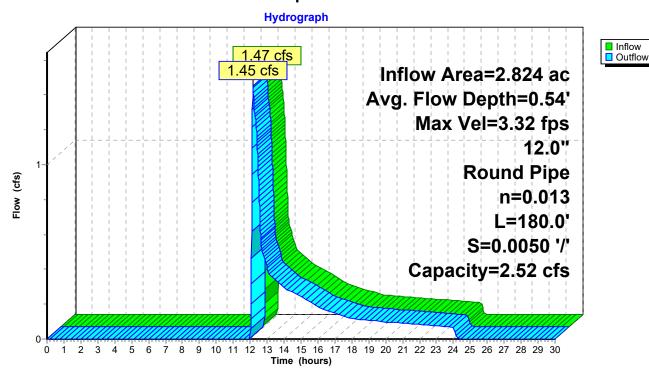
Avg. Velocity = 1.71 fps, Avg. Travel Time= 1.8 min

Peak Storage= 79 cf @ 12.15 hrs Average Depth at Peak Storage= 0.54', Surface Width= 1.00' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe n= 0.013 Length= 180.0' Slope= 0.0050 '/' Inlet Invert= 168.00', Outlet Invert= 167.10'



#### Reach 1R: Prop. Drain DMH 1 to DMH 2



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# Summary for Reach 2R: Prop. Drain DMH 2 to DMH 3

Inflow Area = 2.824 ac, 9.26% Impervious, Inflow Depth = 0.87" for 100-year Storm event

Inflow = 1.45 cfs @ 12.16 hrs, Volume= 0.204 af

Outflow = 1.44 cfs @ 12.19 hrs, Volume= 0.204 af, Atten= 1%, Lag= 1.8 min

Routed to Reach 3R: Prop. Drain DMH 3 to DMH 4

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.31 fps, Min. Travel Time= 0.9 min

Avg. Velocity = 1.68 fps, Avg. Travel Time= 1.8 min

Peak Storage= 78 cf @ 12.18 hrs

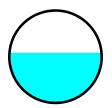
Average Depth at Peak Storage= 0.54', Surface Width= 1.00' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe

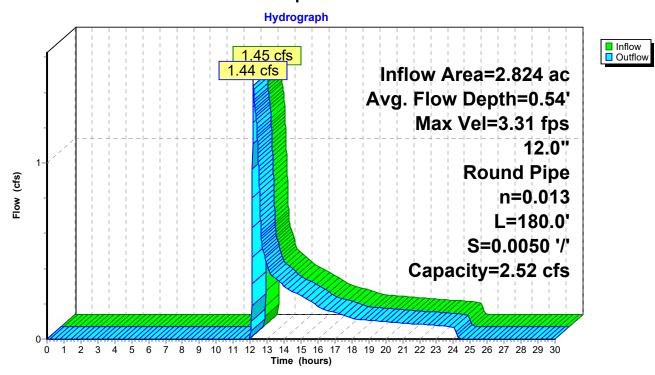
n = 0.013

Length= 180.0' Slope= 0.0050 '/'

Inlet Invert= 167.00', Outlet Invert= 166.10'



#### Reach 2R: Prop. Drain DMH 2 to DMH 3



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## Summary for Reach 3R: Prop. Drain DMH 3 to DMH 4

Inflow Area = 3.638 ac, 9.45% Impervious, Inflow Depth = 0.88" for 100-year Storm event

Inflow = 1.89 cfs @ 12.19 hrs, Volume= 0.268 af

Outflow = 1.88 cfs @ 12.20 hrs, Volume= 0.268 af, Atten= 0%, Lag= 1.0 min

Routed to Reach 4R: Prop Drain DMH 4 to DMH 5

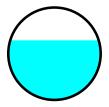
Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 3.52 fps, Min. Travel Time= 0.5 min

Avg. Velocity = 1.80 fps, Avg. Travel Time= 1.0 min

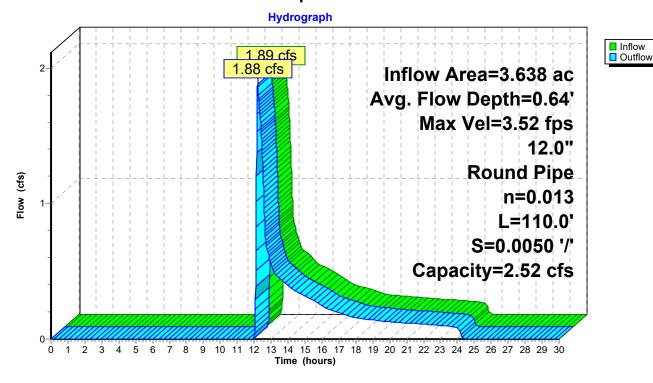
Peak Storage= 59 cf @ 12.20 hrs

Average Depth at Peak Storage= 0.64', Surface Width= 0.96' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe n= 0.013 Length= 110.0' Slope= 0.0050 '/' Inlet Invert= 166.00', Outlet Invert= 165.45'



#### Reach 3R: Prop. Drain DMH 3 to DMH 4



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#### Summary for Reach 4R: Prop Drain DMH 4 to DMH 5

Inflow Area = 3.638 ac, 9.45% Impervious, Inflow Depth = 0.88" for 100-year Storm event

Inflow = 1.88 cfs @ 12.20 hrs, Volume= 0.268 af

Outflow = 1.88 cfs @ 12.21 hrs, Volume= 0.268 af, Atten= 0%, Lag= 0.3 min

Routed to Pond 1P: Recharge System A

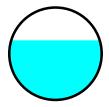
Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 3.52 fps, Min. Travel Time= 0.1 min

Avg. Velocity = 1.80 fps, Avg. Travel Time= 0.3 min

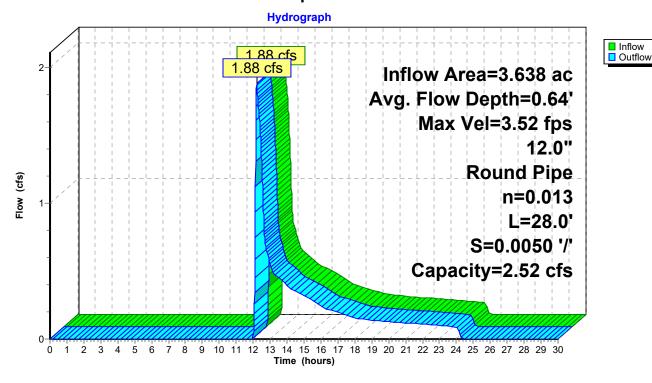
Peak Storage= 15 cf @ 12.21 hrs

Average Depth at Peak Storage= 0.64', Surface Width= 0.96' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 2.52 cfs

12.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 28.0' Slope= 0.0050 '/' Inlet Invert= 165.35', Outlet Invert= 165.21'



#### Reach 4R: Prop Drain DMH 4 to DMH 5



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#### Summary for Reach 5R: Prop. Drain DMH x to DMH x

Inflow Area = 1.060 ac, 39.30% Impervious, Inflow Depth = 3.16" for 100-year Storm event

Inflow = 3.76 cfs @ 12.10 hrs. Volume = 0.279 af

Outflow = 3.76 cfs @ 12.11 hrs, Volume= 0.279 af, Atten= 0%, Lag= 0.3 min

Routed to Pond 3P: Recharge System C

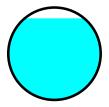
Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 5.17 fps, Min. Travel Time= 0.1 min

Avg. Velocity = 2.16 fps, Avg. Travel Time= 0.3 min

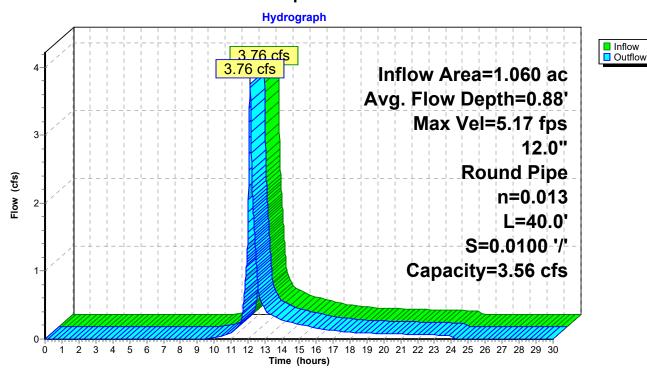
Peak Storage= 29 cf @ 12.11 hrs

Average Depth at Peak Storage= 0.88', Surface Width= 0.65' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 3.56 cfs

12.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 40.0' Slope= 0.0100 '/' Inlet Invert= 164.40', Outlet Invert= 164.00'



#### Reach 5R: Prop. Drain DMH x to DMH x



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# **Summary for Reach DP: Design Point**

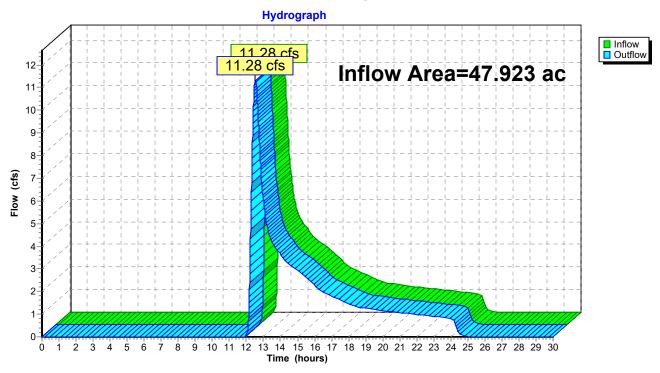
Inflow Area = 47.923 ac, 7.45% Impervious, Inflow Depth = 0.57" for 100-year Storm event

Inflow = 11.28 cfs @ 12.53 hrs, Volume= 2.279 af

Outflow = 11.28 cfs @ 12.53 hrs, Volume= 2.279 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

## **Reach DP: Design Point**



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## Summary for Pond 1P: Recharge System A

Inflow Area = 3.638 ac, 9.45% Impervious, Inflow Depth = 0.88" for 100-year Storm event 1.88 cfs @ 12.21 hrs, Volume= Inflow 0.268 af

0.44 cfs @ 13.61 hrs, Volume= Outflow = 0.268 af, Atten= 77%, Lag= 84.0 min

0.44 cfs @ 13.61 hrs, Volume= Discarded = 0.268 af 0.000 af Primary 0.00 cfs @ 0.00 hrs, Volume=

Routed to Reach DP : Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 167.04' @ 13.61 hrs Surf.Area= 1,309 sf Storage= 2,557 cf

Plug-Flow detention time= 60.8 min calculated for 0.268 af (100% of inflow)

Center-of-Mass det. time= 60.7 min ( 991.6 - 930.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	164.50'	1,608 cf	ADS_StormTech SC-740 b +Cap x 35 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	164.00'	1,189 cf	25.25'W x 51.84'L x 3.50'H Prismatoid
			4,581 cf Overall - 1,608 cf Embedded = 2,973 cf x 40.0% Voids

2,797 cf Total Available Storage

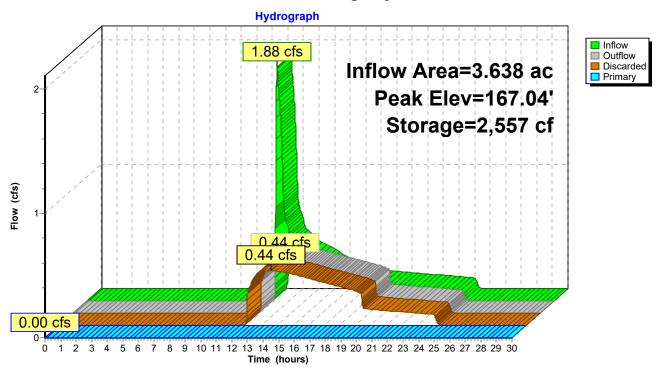
Device	Routing	Invert	Outlet Devices
#1	Discarded	164.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 160.00'
#2	Primary	171.25'	6.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

**Discarded OutFlow** Max=0.44 cfs @ 13.61 hrs HW=167.04' (Free Discharge) 1=Exfiltration (Controls 0.44 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=164.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

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# Pond 1P: Recharge System A



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## Summary for Pond 2P: Recharge System B

Inflow Area = 7.186 ac, 9.69% Impervious, Inflow Depth = 1.06" for 100-year Storm event 
Inflow = 5.59 cfs @ 12.13 hrs, Volume= 0.636 af 
Outflow = 1.42 cfs @ 12.82 hrs, Volume= 0.636 af, Atten= 75%, Lag= 41.6 min 
Discarded = 1.42 cfs @ 12.82 hrs, Volume= 0.636 af 
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach DP: Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 164.57' @ 12.82 hrs Surf.Area= 3,257 sf Storage= 5,718 cf

Plug-Flow detention time= 37.4 min calculated for 0.636 af (100% of inflow) Center-of-Mass det. time= 37.4 min (951.0 - 913.6)

Volume	Invert	Avail.Storage	Storage Description
#1	162.50'	4,135 cf	ADS_StormTech SC-740 b +Cap x 90 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	162.00'	2,907 cf	44.50'W x 73.20'L x 3.50'H Prismatoid
			11,401 cf Overall - 4,135 cf Embedded = 7,266 cf x 40.0% Voids

7,041 cf Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	162.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 160.00'
#2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600
	_		I imited to weir flow at low heads

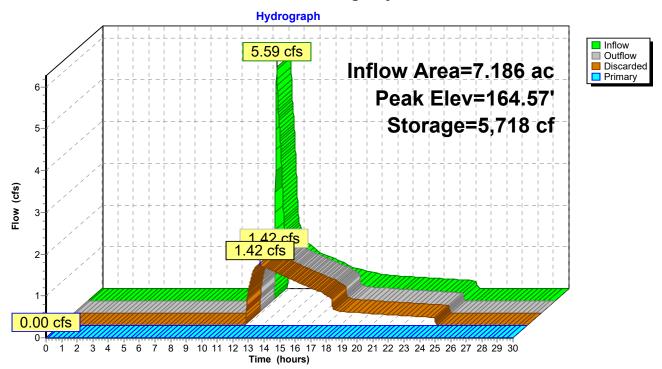
Discarded OutFlow Max=1.42 cfs @ 12.82 hrs HW=164.57' (Free Discharge)
1=Exfiltration (Controls 1.42 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=162.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

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# Pond 2P: Recharge System B



#### Pleasant Street Dunstable Post-Development-Type III 24-hr 100-year Storm Rainfall=7.48"

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## Summary for Pond 3P: Recharge System C

Inflow Area = 1.060 ac, 39.30% Impervious, Inflow Depth = 3.16" for 100-year Storm event

Inflow = 3.76 cfs @ 12.11 hrs, Volume= 0.279 af

Outflow = 0.86 cfs (a) 12.55 hrs, Volume= 0.279 af, Atten= 77%, Lag= 26.7 min

Discarded = 0.86 cfs @ 12.55 hrs, Volume= 0.279 af Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach DP: Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 164.84' @ 12.55 hrs Surf.Area= 1,848 sf Storage= 3,470 cf

Plug-Flow detention time= 34.5 min calculated for 0.279 af (100% of inflow)

Center-of-Mass det. time= 34.5 min ( 880.8 - 846.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	162.50'	2,297 cf	ADS_StormTech SC-740 b +Cap x 50 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	162.00'	1,669 cf	25.25'W x 73.20'L x 3.50'H Prismatoid
			6,469 cf Overall - 2,297 cf Embedded = 4,172 cf x 40.0% Voids

3,966 cf Total Available Storage

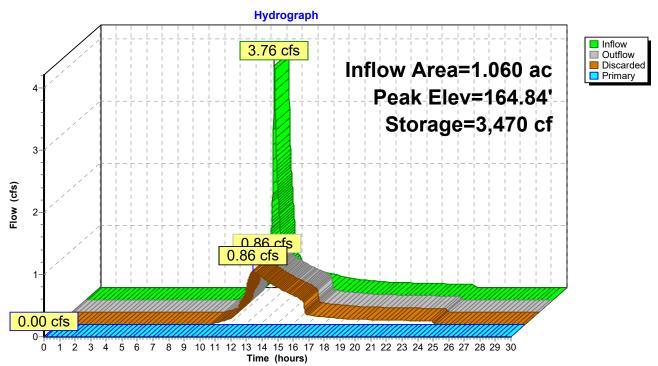
Device	Routing	Invert	Outlet Devices
#1	Discarded	162.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 160.00'
#2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

**Discarded OutFlow** Max=0.86 cfs @ 12.55 hrs HW=164.84' (Free Discharge) 1=Exfiltration (Controls 0.86 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=162.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

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# Pond 3P: Recharge System C



# Pleasant Street Dunstable Post-Development-Type III 24-hr 100-year Storm Rainfall=7.48"

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## Summary for Pond 4P: Recharge System D

Inflow Area = 1.273 ac, 41.36% Impervious, Inflow Depth = 3.26" for 100-year Storm event

Inflow = 4.64 cfs @ 12.11 hrs, Volume= 0.346 af

Outflow = 1.07 cfs @ 12.55 hrs, Volume= 0.346 af, Atten= 77%, Lag= 26.5 min

Discarded = 1.07 cfs @ 12.55 hrs, Volume= 0.346 af Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach DP: Design Point

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 164.41' @ 12.55 hrs Surf.Area= 2,544 sf Storage= 4,214 cf

Plug-Flow detention time= 31.4 min calculated for 0.346 af (100% of inflow)

Center-of-Mass det. time= 31.4 min (875.5 - 844.0)

Volume	Invert	Avail.Storage	Storage Description
#1	162.50'	3,216 cf	ADS StormTech SC-740 b +Cap x 70 Inside #2
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
#2	162.00'	2,275 cf	34.75'W x 73.20'L x 3.50'H Prismatoid
			8,903 cf Overall - 3,216 cf Embedded = 5,687 cf x 40.0% Voids

5,491 cf Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	162.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 160.00'
#2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600
	-		I imited to weir flow at low heads

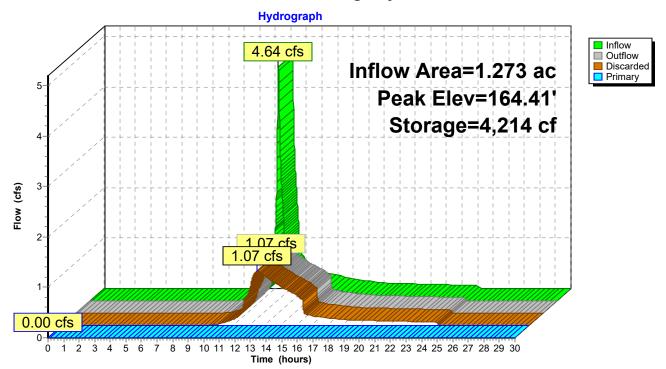
**Discarded OutFlow** Max=1.07 cfs @ 12.55 hrs HW=164.41' (Free Discharge) **1=Exfiltration** (Controls 1.07 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=162.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

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# Pond 4P: Recharge System D



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### Summary for Pond RAP1: Typical Roof Recharge Area 1

Inflow Area = 0.030 ac,100.00% Impervious, Inflow Depth = 7.24" for 100-year Storm event 
Inflow = 0.22 cfs @ 12.08 hrs, Volume= 0.018 af 
Outflow = 0.05 cfs @ 12.47 hrs, Volume= 0.018 af, Atten= 77%, Lag= 23.4 min 
Discarded = 0.00 cfs @ 12.47 hrs, Volume= 0.018 af 
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 165.67' @ 12.47 hrs Surf.Area= 187 sf Storage= 177 cf

Plug-Flow detention time= 18.9 min calculated for 0.018 af (100% of inflow) Center-of-Mass det. time= 18.9 min (761.0 - 742.1)

Volume	Invert	Avail.Storage	Storage Description	
#1	164.50'	88 cf	ADS StormTech SC-310 x 6 Inside #2	
			Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf	
			Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap	
#2	164.00'	139 cf	11.50'W x 16.24'L x 2.33'H Prismatoid	
			435 cf Overall - 88 cf Embedded = 347 cf x 40.0% Voids	
		227 cf	Total Available Storage	

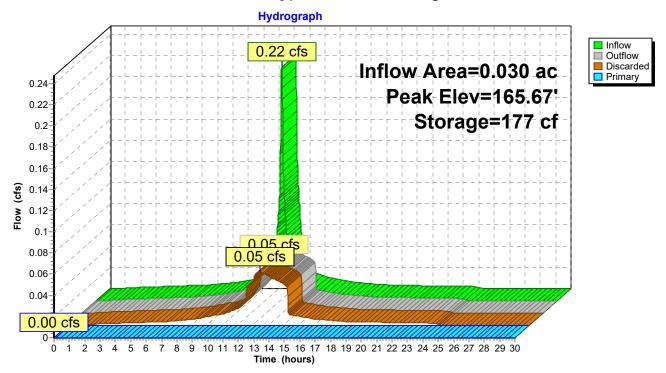
Device	Routing	Invert	Outlet Devices
#1	Discarded	164.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 160.00'
#2	Primary	166.50'	6.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

Discarded OutFlow Max=0.05 cfs @ 12.47 hrs HW=165.67' (Free Discharge) 1=Exfiltration (Controls 0.05 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=164.00' (Free Discharge) 2=Orifice/Grate ( Controls 0.00 cfs)

### Pond RAP1: Typical Roof Recharge Area 1

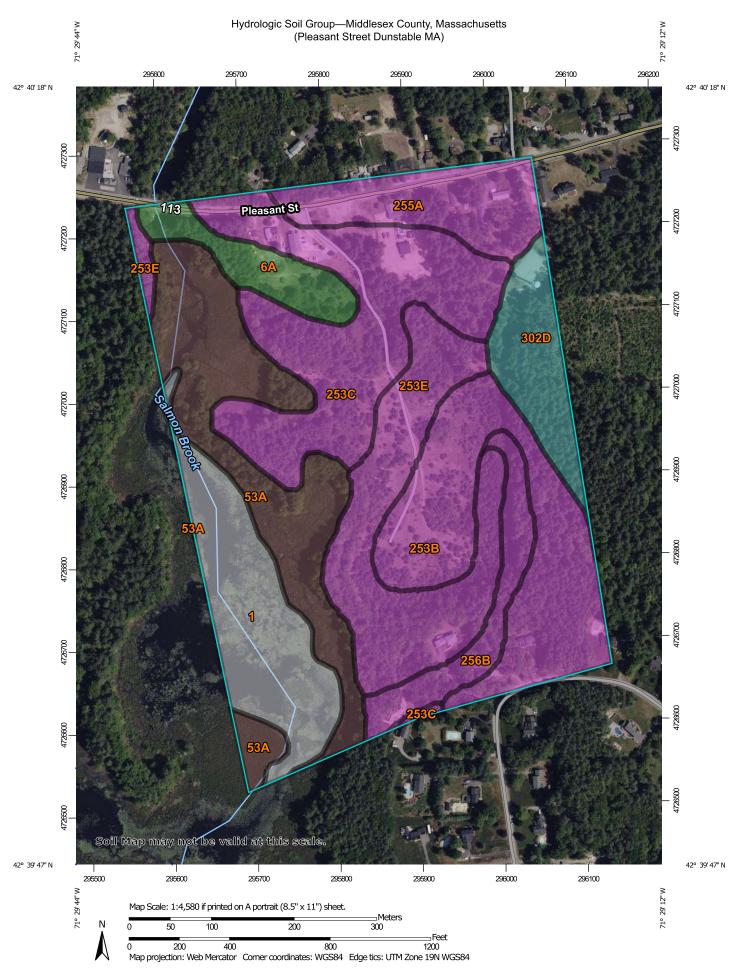
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# FIGURE 1 PRE-DEVELOPMENT WATERSHED DELINEATION

# FIGURE 2 POST-DEVELOPMENT WATERSHED DELINEATION

# FIGURE 3 SOILS MAP



National Cooperative Soil Survey Web Soil Survey

USDA



## **Hydrologic Soil Group**

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
1	Water		8.9	11.3%
6A	Scarboro mucky fine sandy loam, 0 to 3 percent slopes	A/D	3.3	4.2%
53A	Freetown muck, ponded, 0 to 1 percent slopes	B/D	13.0	16.5%
253B	Hinckley loamy sand, 3 to 8 percent slopes	А	12.5	15.9%
253C	Hinckley loamy sand, 8 to 15 percent slopes	А	14.9	18.9%
253E	Hinckley loamy sand, 25 to 35 percent slopes	А	12.0	15.2%
255A	Windsor loamy sand, 0 to 3 percent slopes	А	5.8	7.4%
256B	Deerfield loamy fine sand, 3 to 8 percent slopes	A	3.7	4.7%
302D	Montauk fine sandy loam, 15 to 35 percent slopes, extremely stony	С	4.6	5.9%
Totals for Area of Inter	rest	78.7	100.0%	

### **Description**

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

### **Rating Options**

Aggregation Method: Dominant Condition

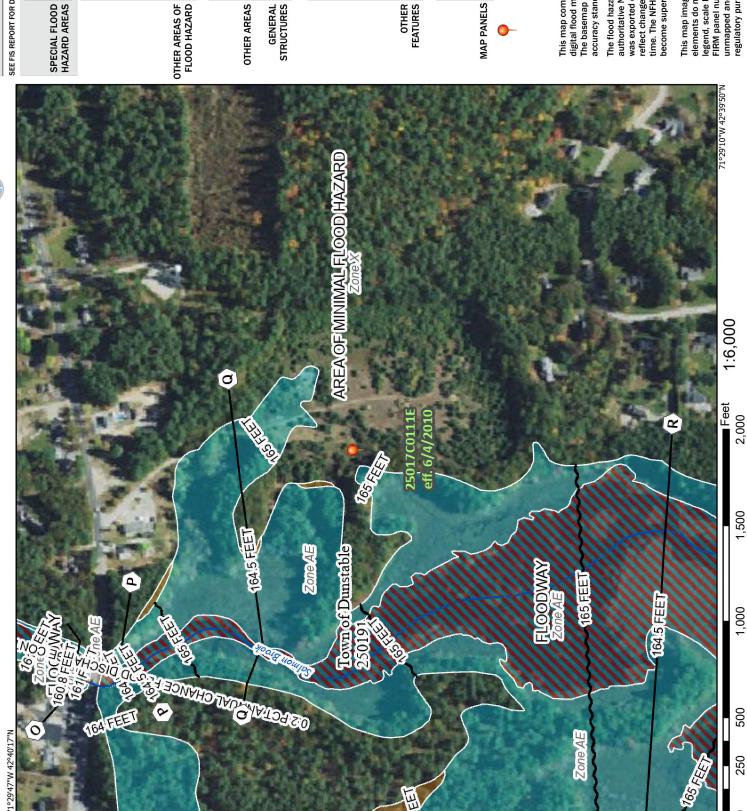
Component Percent Cutoff: None Specified

Tie-break Rule: Higher

# FIGURE 4 FLOOD INSURANCE RATE MAP

# National Flood Hazard Layer FIRMette





# **Legend**

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

Without Base Flood Elevation (BFE)

With BFE or Depth Zone AE, AO, AH, VE, AR Regulatory Floodway

0.2% Annual Chance Flood Hazard, Areas depth less than one foot or with drainage areas of less than one square mile Zone X of 1% annual chance flood with average

Area with Reduced Flood Risk due to Future Conditions 1% Annual Chance Flood Hazard Zone X Levee. See Notes. Zone X

Area with Flood Risk due to Levee Zone D

NO SCREEN Area of Minimal Flood Hazard Zone X **Effective LOMRs** 

Area of Undetermined Flood Hazard Zone D

Channel, Culvert, or Storm Sewer STRUCTURES | 1111111 Levee, Dike, or Floodwall

Cross Sections with 1% Annual Chance Water Surface Elevation Coastal Transect (B) 20.2

Base Flood Elevation Line (BFE) Limit of Study me 513 mm

Jurisdiction Boundary

Coastal Transect Baseline

Hydrographic Feature

OTHER FEATURES

Digital Data Available

No Digital Data Available Unmapped

point selected by the user and does not represent an authoritative property location. The pin displayed on the map is an approximate

This map complies with FEMA's standards for the use of The basemap shown complies with FEMA's basemap digital flood maps if it is not void as described below accuracy standards

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<sup>&</sup>lt;sup>1</sup>"Urban Hydrology for Small Watersheds (Technical Release Number 55); Engineering Division, United States Dept. of Agriculture, Soil Conservation Service (Jan. 1975).

<sup>&</sup>lt;sup>2</sup>"National Engineering Handbook Section 4-Hydrology"; United States Dept. of Agriculture, Soil Conservation Service (March 1985)

<sup>&</sup>lt;sup>3</sup>"HydroCAD" Drainage software developed by Applied Microcomputer, Page Hill Road, Chocorua, NH; Tel. (604) 927-RAIN Copyrights 1986-1992. (version 3.02)